- BFS 2011:6 WITH AMENDMENTS UP TO AND INCLUDING BFS 2017:5. 2. BOVERKET'S MANDATORY PROVISIONS AND GENERAL RECOMMENDATIONS ON THE APPLICATION OF EUROPEAN DESIGN
- STANDARDS (EUROCODES) EKS 10, BFS 2011:10 WITH AMENDMENTS UP TO AND INCLUDING BFS 2015:6. VALID EUROCODES.
- 4. NORMATIVE STANDARDS ACCORDING TO RESPECTIVELY EUROCODE. 5. DETAILS SHALL BE ACCORDING TO AMA HUS 14 AND RA HUS 13 IF NOTHING ELSE IS STATED.
- 6. DETAILS SHALL BE ACCORDING TO AMA ANLÄGGNING 10 AND RA ANLÄGGNING 10 IF NOTHING ELSE IS STATED

ENVIRONMENTAL EVALUATION

ALL USED BUILDING MATERIAL MUST BE ENVIRONMENTALLY APPROVED BY DOCUMENTATION AND VALIDATION WITH RESPECT TO COMPONENT PARTS, ADDITIVES AND MANUFACTURE ACCORDING TO BYGGVARUBEDÖMNINGEN AND IN SOME PLACES BE VALIDATED WITH RESPECT TO RADIATION.

- SK 3 MAIN STRUCTURAL SYSTEM OF LOAD BEARING MEMBERS AND SECONDARY STRUCTURES PROVIDING NECESSARY BRACING FOR MAIN STRUCTURE. BEAMS, COLUMNS, SLABS, WALLS AND STAIRS IN ESCAPE ROUTES.
- SK 2 FOUNDATIONS, RETAINING WALLS, HEAVY EXTERNAL WALLS AND CURTAIN WALLS.
- SK 1 FLOOR SLABS ON GROUND.

ACCIDENTAL ACTION AND PROGRESSIVE COLLAPSE

REGARDING ACCIDENTAL ACTIONS AND PROGRESSIVE COLLAPS EUROCODE SS-EN 1991-1-6 (ACTIONS DURING EXECUTION) AND SS-EN 1991-1-7 (ACCIDENTAL ACTIONS) SHALL BE CONSIDERED. DESIGN FOR CONSEQUENCES OF LOCALISED FAILURE IN BUILDING FROM AN UNSPECIFIED CAUSE IS DONE WITH THE STRATEGY USING VERTICAL AND HORISONTAL TIES DESCRIBED IN SS-EN 1991-1-7 ANNEX A.

CONSEQUENCE CLASS CC3, RISKANALYSIS SHALL BE PERFORMED IN

DESIGN WORKING LIFE AND DURABILITY

DETAIL DESIGN PHASE.

LOAD BEARING STRUCTURES IN SAFETY CLASS 2 AND 3 SHOULD BE DESIGNED FOR A DESIGN WORKING LIFE CATEGORY OF:

- 4 (50 YEARS) IF THE MEMBERS ARE ACCESSIBLE FOR INSPECTION AND MAINTENANCE.
- -5 (100 YEARS) IF THE MEMBERS ARE NOT ACCESSIBLE FOR INSPECTION AND MAINTENANCE.

FURTHERMORE SHALL RECOMMENDATIONS IN EOTA GUIDANCE DOCUMENT 002 APPLY FOR ALL STRUCTURAL MEMBERS REGARDING ITS

REPLACEABILITY AND REPAIRABILITY. MEMBERS IN DESIGN WORKING LIFE CATEGORY 4 FOLLOW EOTA CATEGORY

MEMBERS IN DESIGN WORKING LIFE CATEGORY 5 FOLLOW EOTA CATEGORY "LONG".

ALL OTHER SHOULD FOLLOW EOTA GUIDANDE DOCUMENT 002 CATEGORY "NORMAL"

SAFETY IN CASE OF FIRE

BUILDING CLASS: Br2

ALL STRUCTURES SUPPORTING EI60 FIRE COMPARTMENTS IN ALL AREAS

SEE ALSO FIRE SAFETY STRATEGY REPORT, ESS-0002381

MOISTURE

ALL BUILDING MATERIALS SENSITIVE TO MOISTURE OR WITH LIMITED POSSIBILITY TO DRY SHOULD BE PROTECTED AGAINST MOISTURE DURING CONSTRUCTION.

STABILITY

THE TALL PART OF THE BUILDING IS STABILIZED WITH CONCRETE SLABS, THAT SHALL BE DESIGNED AS HORIZONTAL LOAD BEARING AND BE ABLE TRANSFER THE STABILITY LOADS TO THE STABILIZING CONCRETE WALLS, BOTH EXTERIOR AND INTERIOR.

THE LOWER PART OF THE BUILDING IS STABILIZED WITH A STEEL SHEETING, THAT SHALL BE DESIGNED AS HORIZONTAL LOAD BEARING WITH SUFFICIENT STIFFNESS TO TRANSFER THE STABILITY LOADS TO THE STEEL WIND BRACINGS AND THE CONCRETE WALLS. THE CONTRACTOR IS RESPONSIBLE OF NECESSARY BRACING, ANCHORING AND ATTACHMENTS DURING EXECUTION. THEY SHALL PREPARE A METHOD

DEFORMATIONS / DEFLECTIONS

SECTIONS.

STATEMENT SUBJECT TO THE CLIENT APPROVAL.

DEFLECTIONS FOR LOAD BEARING STRUCTURES SHOULD BE LIMITED ACCORDING TO THE FOLLOWING TABLE. DEFLECTION CONTROL SHALL BE APPLIED AS A SERVICEABILITY LIMIT STATE WITH FREQUENT LOAD COMBINATION (6.15b). DEFINITIONS ARE AS STATED IN FIGURE A1.1 OCH A1.2 SS-EN 1990. CONCRETE STRUCTURES SHALL BE CALCULATED WITH CRACKED CROSS-

ROOF CLADDING $w_2 + w_3 = L/150$ ROOF BEAMS AND GIRDERS $w_2 + w_3 = L/300$ WALKWAY AND STAIR BEAMS $w_2 + w_3 = L/300$ $w_2 + w_3 = L/400$ SLABS AND BEAMS IN GENERAL SLABS AND BEAMS SUPPORTING WALLS $w_2 + w_3 = L/500$ BEAMS SUPPORTING COLUMNS $w_2 + w_3 = L/500$ BEAMS SUPPORTING MASONRY $w_2 + w_3 = L/1000$ GRATING $w_2 + w_3 = L/200$ COLUMNS u = H/500, $u_i = H_i/300$

CONDITIONS OF EXECUTION

COLUMNS ARE UNDERPOURED BEFORE WALLS, SLABS AND ROOF IS ASSEMBLED. STABILIZING ROOF METAL SHEET AND WIND STIFFENERS ARE ASSEMBLED BEFORE WALLS ARE ASSEMBLED. ASSEMBLIES SHALL FOLLOW THE CONTRACTORS PLAN OR INSTRUCTION

OF ERECTION.

FOR OPENINGS/HOLES NOT STATED IN THE DESIGN DRAWINGS THE FOLLOWING APPLY:

HOLES/OPENINGS IN ROOF, SLABS, CONCRETE WALLS, COLUMNS, BEAMS AND BEAM SUPPORTS MAY NOT BE EXECUTED WITHOUT CONSULTATION OF THE RESPONSIBLE STRUCTURAL ENGINEER. OVER CUTTING IS NOT ALLOWED DURING CUTTING IN CONCRETE

GROUNDING AND LIGHTNING PROTECTION

ALL BUILDINGS SHALL BE EQUIPPED WITH LIGHTNING PROTECTION AND POTENTIAL EQUALIZATION. CONNECTION OF STEEL FRAMEWORK, REINFORCING STEEL, STEEL STUDS AND EMBEDMENT OF GROUNDING CONDUCTOR ACCORDING TO E-DRAWINGS.

<u>MATERIALS</u>

STRUCTURES.

ALL MATERIALS AND ALL SURFACES IN THE CONTROLLED AREA SHALL BE CHOSEN ACCORDING TO PAKT / TBY - "TECHNICAL REGULATIONS FOR SURFACE TREATMENT" IN ORDER TO EASE DECONTAMINATION. FOR MOISTURE LEVELS, SEE CONCRETE STRUCTURES, EXECUTION ON K07---0-H09---002.

1 LOAD ACTIONS ON STRUCTURES

LOAD CONDITIONS

ACTIONS DURING EXECUTION: ACCORDING TO SS-EN 1991-1-6

<u>SNOW:</u>	SNOW ZONE 1,5 s _k =1,5 kN/m ² EXPOSURE COEFFICIENT SHAPE COEFFICIENT	Ψ_0 =0,6; Ψ_1 =0,3; Ψ_2 =0,1 C_e =1,0 ACCORDING TO SS-EN 1991-1-3.
WIND:	BASIC WIND VELOCITY	$v_b=26$ m/s, TERRAIN CATEGORY I $\Psi_0=0.3$; $\Psi_1=0.2$; $\Psi_2=0$
	PRESSURE COEFFICIENT	ACCORDING TO

SS-EN-1991-1-4.

DEAD LOAD:

STRUCTURAL MEMBER	STRUCTURE kN/m²	INSTALLATIONS kN/m ²	MOVABLE PARTITIONS/ WALLS kN/m ²	TOT kN/i
SLAB BJK01	5,60	0,50	0,80	6,90
SLAB BJK02	5,00	0,50	0,80	6,30
SLAB BJK03	5,00	0,50		5,50
SLAB BJK04	7,70	0,50	0,80	9,00
R00F T01	5,00	0,50		5,50
R00F T02	0,60	0,50		1,10
R00F T03	7,50	0,50		8,00
OUTER WALL YV01	7,80			7,80
INNER WALL IVB01	7,50			7,50

SEE LOAD PLANS, K-07-01---1-H09100110, K-07-01---1-H09110110, K-07-01---1-H09115110 AND K-07-01---1-H09120110.

ATB 12K VEHICLE ACCORDING TO ESS-0149584 SHALL BE COMBINATED WITH FORKLIFT.

OVERHEAD CRANES

OVERHEAD CRANES WITH CAPACITIES ACCORDING TO ESS-0046980 SHALL BE CONSIDERED. LOADS AND OPERATING CLASSES OF THE CRANES SHALL BE ACQUIRED FROM TRANSPORT SYSTEM OWNERS IN DETAIL DESIGN PHASE.

ACCORDING TO SS-EN 1991-1-1 CHAPTER 6.3.2.3 CLASS OF FORKLIFT: FL5 AXLE LOAD Q_K=140 kN NET WEIGHT W=90 kN

ACCIDENTAL ACTIONS FROM FORKLIFTS

ACCORDING TO SS-EN 1991-1-7, EKS 8 AND SS-EN 1991-1-1 TABLE 6.5. $F_d = 5*W$ IMPACT HEIGHT = 0,75 m ABOVE FLOOR LEVEL.

ACCIDENTAL ACTIONS FROM LOAD DROPS

ACCORDING TO DESIGN MANUAL, CONVENTIONAL FACILITIES ESS-0001497 CHAPTER 5.3.1.3.

$F_k = 10 \text{ kN}$

ACCIDENTAL ACTIONS FROM CRANES

LOADS SHALL BE ACQUIRED FROM TRANSPORT SYSTEM OWNERS IN DETAIL DESIGN PHASE

EXCEPTIONAL INTERNAL WATER PRESSURE WATER PRESSURE ON WALLS IN TANK ROOM, IN CASE OF INTERNAL FLOODING, SHALL BE CONSIDERED.

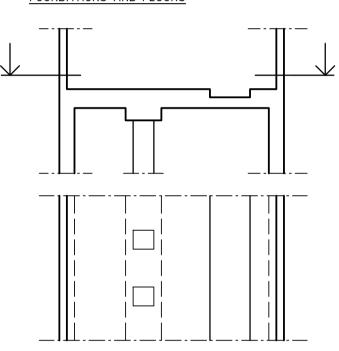
ASSUMED MAXIMUM WATER LEVEL: 1500mm ABOVE FFL.

SYMBOLS

	PRECAST CONCRETE
	CAST-IN-SITU CONCRETE
	WATERTIGHT CONCRETE
4 44	EXISTING CONCRETE
	LIGHTWEIGHT AGGREGATE CONCRETE
	CLAY BRICKS
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	DRAINAGE LAYER
	MINERAL WOOL
	CELLULAR PLASTIC, EPS OR XPS
00000000	ISODRÄN OR EQUAL
7	REINFORCEMENT BARS IN UPPER SURFACE
∠ <u></u>	REINFORCEMENT BARS IN LOWER SURFACE
	NEW DRAINAGE PIPE
	MAJOR-AXIS / REINFORCEMENT MAIN DIRECTION

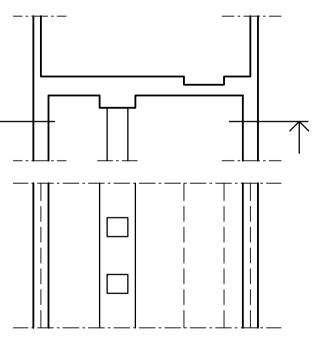
PLAN DRAWINGS

FOUNDATIONS AND FLOORS



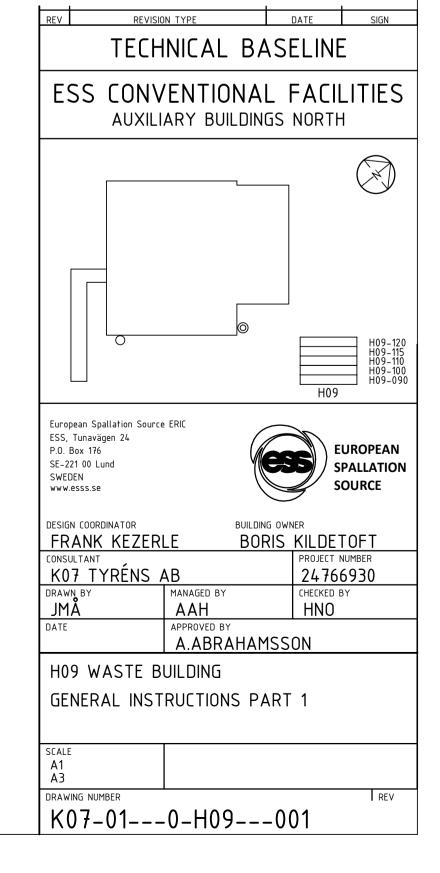
DRAWINGS ARE PERFORMED WITH DIRECT PARALLEL PROJECTION

SLABS



DRAWINGS ARE PERFORMED WITH MIRRORED PARALLEL PROJECTION

PD DRAFT 2018-01-09



SCALE 1:100 A1

EXPOSURE CLASSES AS BELOW ARE APPLICABLE TO CONCRETE AND REINFORCEMENT ACCORDING TO SS-EN 206-1.

FLOOR SLABS ON GROUND:

IVB01A, IVB01B)

G01, OVERHEAD CRANE HALL	
EXPOSURE CLASS, BOTTOM SIDE	XC3
EXPOSURE CLASS, UPPER SIDE	XF3
STRENGTH CLASS	C35/45
CURING CLASS	4
TIGHTNESS CLASS	1
VCT	<=0,40
DESIGN WORKING LIFE	L100
CONCRETE COVER AGAINST INSULATION	LARGEST OF 35mm AND Ø+15mm
CONCRETE COVER AGAINST SURFACE	LARGEST OF 55mm AND Ø+10mm

G01, OTHERWISE (INCLUDING ALSO WALL BASES STANDING ON GO1, I.E. YVO1A, YVO1B,

EXPOSURE CLASS	XC3
STRENGTH CLASS	C30/37
CURING CLASS	4
TIGHTNESS CLASS	1
VCT	<=0,55
DESIGN WORKING LIFE	L100
CONCRETE COVER AGAINST INSULATION	LARGEST OF 40mm AND Ø+15mm
CONCRETE COVER AGAINST SURFACE	LARGEST OF 35mm AND Ø+10mm

G02		
EXPOSURE CLASS	XC3	
STRENGTH CLASS	C30/37	
TIGHTNESS CLASS	1	

VCT		<=0,55				
CURING CLASS		4				
DESIGN WORKING LIFE		L100				
CONCRETE COVER AGAINST	INSULATION	LARGEST	OF	40mm	AND	Ø+15mm
CONCRETE COVER AGAINST	SURFACE	LARGEST	OF	35mm	AND	Ø+10mm

G03	
EXPOSURE CLASS	XC3
STRENGTH CLASS	C30/37
VCT	<=0,55
CURING CLASS	3
DESIGN WORKING LIFE	L100
CONCRETE COVER AGAINST INSULATION	LARGEST OF 40mm

mm AND Ø+15mm CONCRETE COVER AGAINST SURFACE LARGEST OF 35mm AND Ø+10mm BASEMENT WALL, KYV01:

27132112111 117122, 111 V 0 11	
EXPOSURE CLASS	XC4+XF3
STRENGTH CLASS	C30/37
TIGHTNESS CLASS	1
VCT	<=0,50
CURING CLASS	4
DESIGN WORKING LIFE	L100
CONCRETE COVER	LARGEST OF 30mm AND Ø+1
CIII VERT	

CULVERT	
EXPOSURE CLASS	XC4+XF3
STRENGTH CLASS	C30/37
TIGHTNESS CLASS	1
VCT	<=0,50
CURING CLASS	4
DESIGN WORKING LIFE	L100
CONCRETE COVER AGAINST INSULATION	LARGEST OF 40mm AND Ø+15

CONCRETE COVER AGAINST SURFACE	LARGEST OF 35mm AND Ø+10mm
BASE ELEMENTS:	

EXPOSURE CLASS	XD3+XF4
STRENGTH CLASS	C35/45, FREEZE/THAW
	RESISTANT CONCRETE ACCORDING
	TO SS137244.
VCT	<=0,40
CURING CLASS	3

DESIGN WORKING LIFE	L50
CONCRETE COVER	LARGEST OF 45mm AND Ø+10mm

STACK FOUNDATION:	
EXPOSURE CLASS	XD3+XF4
STRENGTH CLASS	C35/45, FREEZE/THAW
	RESISTANT AGGREGATE
VCT	<=0,40
CURING CLASS	4

AIR CONTENT			MIN 4,5%					
DMAX			16mm					
CONCRETE COVER	AGAINST	GROUND	LARGEST	OF	40mm	${\sf AND}$	Ø+15mm	
CONCRETE COVER	AGAINST	SURFACE	LARGEST	0F	55mm	AND	Ø+10mm	

L100

INTERNAL CONCRETE STRUCTURES:	
EXPOSURE CLASS	XC1
STRENGTH CLASS	C25/30
VCT	<=0,60
CURING CLASS	3

DESIGN WORKING LIFE

DESIGN WORKING LIFE L50 LARGEST OF 20mm AND Ø+10mm CONCRETE COVER

THE MAXIMUM CRACK WIDTH WK SHALL, IN SERVICEABILITY LIMIT STATE AND IN QUASI PERMANENT SITUATIONS, BE LIMITED TO THE LOWEST OF 0,3 MM OR THE VALUE IN TABLE D-2 IN EKS 9.

REINFORCEMENT MESH REINFORCEMENT: NK500B, NK500AB OTHER REINFORCEMENT: K500C, K500B

REINFORCEMENT LAPS

LAPS BETWEEN BARS SHOULD BE STAGGERED SO THAT NOT MORE THAN HALF OF THE REINFORCEMENT ARE LAPPED IN THE SAME SECTION. THE LONGITUDINAL DISTANCE BETWEEN TWO ADJACENT LAPS SHOULD NOT BE LESS THAN 0,3 TIMES THE LAP LENGTH, SEE SS-EN 1992-1-1 ARTICLE 8.7.2. REINFORCEMENT DESIGN LAP LENGTH ACCORDING TO THE TABLE BELOW.

BAR SIZE	201101111	EINFORCEMENT VERTICAL IN		DRCEMENT IN RISONTALLY		
	C30/37	C 35/45	C30/37	C35/45		
Ø10 Ø12 Ø16 Ø20 Ø25	550 660 870 1090 1360	500 600 800 990 1240	780 940 1250 1560 1950	710 850 1130 1420 1770		

FOR WELDED MESH FABRICS THE LAPPING ARRANGEMENTS SHOULD BE PERFORMED BY OVERLAP JOINT WITH AT LEAST TWO TRANSVERSE BARS IN ONE MESH OVERLAPPING THE OUTER BAR IN THE OTHER. THE OVERLAP JOINT SHOULD BE AT LEAST ACCORDING TO THE TABLE BELOW.

11	>20	//	<u>-20</u>		
$\frac{0}{\sqrt{2}}$	-0	$\frac{3}{2}$			_
Ĭ	O	٠	O	J	
	>200				

BAR SIZE		REINFORCEMENT IN IN SLABS, SLABS, VERTICAL HORISONTALLY IN						
	C30/37	C35/45	C30/37	C35/45				
Ø5 Ø6 Ø7	200 220 270	200 200 250	240 310 380	210 280 350	1 1 2			
Ø8	310	280	440	400	2			
Ø9 Ø10	350 380	320 350	490 540	450 500	2			

BARS ARE BENT ACCORDING TO SS-EN 13670.

BEND RADIUS:

STIRRUPS FOR BARS EXCEPT STIRRUPS (LARGE RADIUS) (STANDARD RADIUS) AT C/C MIN 75 mm, NOT ADJACENT TO A CONCRETE SURFACE.

		C25/30	C30/37	C35/45	C40/50
ø10	24	100	100	64	64
ø12	24	125	100	100	64
ø16	32	160	160	125	100
ø20	100	250	200	160	160
ø25	100	320	250	250	200

CONSTRUCTION JOINTS

CONSTRUCTION JOINTS IN WATERTIGHT STRUCTURES SHALL BE PROVIDED WITH WATER BARRIERS. JOINTS SHALL BE ARRANGED TO AVOID SHRINKAGE CRACKS AND TO MEET THE CRACK WIDTH REQUIREMENTS FOR EACH STRUCTURAL ELEMENT. JOINTS IN FORKLIFT AFFECTED AREAS SHOULD BE DONE AS A FORMED FREE-MOVEMENT JOINT WITH A PROPRIETARY SYSTEM WITH ARRIS PROTECTION AND PLATE DOWEL CONFIGURATION.

SPECIFIED CONCRETE QUALITIES ARE VALID AS MINIMUM QUALITY REGARDING STRENGTH AND EXPOSURE CLASSES.

MOISTURE LEVEL IN CONCRETE MUST NOT EXCEED 85% RH WHEN TIGHT COATING OR PAINT ARE APPLIED.

93% RH WHEN SURFACE MATERIALS ARE APPLIED. MEASURING PROCESS IS CARRIED OUT ACCORDING TO METHODS DESCRIBED

ACCORDING TO TBY, MOISTURE LEVEL IN CONCRETE MUST NOT EXCEED

IN AMA HUS 11 YSC.1. FOR STRUCTURES TO BE TREATED WITH TIGHT COATING THE CONCRETE QUALITY MUST BE SELECTED, IN AGREEMENT WITH THE CLIENT AND WITH RESPECT TO EXECUTION TIME SCHEDULE, SO THAT MOISTURE LEVEL AS ABOVE CAN BE ACHIEVED.

BESIDE OPENINGS WHERE REINFORCEMENT MUST BE CUT OFF, IF OTHERWISE NOT SPECIFIED, THE REINFORCEMENT MUST BE COMPLETED WITH AT LEAST THE SAME AMOUNT AS THE CUT OFF REINFORCEMENT THIS ADDITIONAL REINFORCEMENT SHALL BE DISTRIBUTED EQUAL ON BOTH SIDES OF THE OPENING AND BE EXTENDED WITH AT LEAST A DOUBLE LAP LENGTH.

FORMWORK

ALL OUTWARDS CORNERS SHALL BE PROVIDED WITH INSERTED INTERNAL FILLETS TO FORM CHAMFERED CORNERS UNLESS OTHERWISE STATED.

SAFETY REINFORCEMENT AND VERTICAL STRUTTING ALL CONCRETE WALLS THAT DURING ANY CONSTRUCTION PHASE ARE STANDING BY THEMSELVES MUST AT LEAST BE PROVIDED WITH SAFETY REINFORCEMENT.

VERTICAL SAFETY STRUTTING OF SLABS MUST CONTINUE UNTIL THE CONCRETE ACHIEVED FULL STRENGTH OR AFTER DISCUSSIONS WITH THE STRUCTURAL ENGINEER.

PRECAST CONCRETE

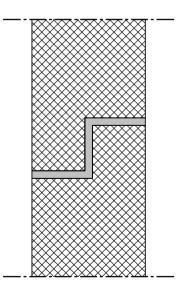
PRECAST ELEMENTS IN GENERAL

SS-EN 13369, SS 137005, RELEVANT PRODUCT STANDARDS AND CEN/TR 15728 APPLIES AS REGULATION. FURTHERMORE SS-EN 17660 APLLIES TO REINFORCEMENT WELDING AND SS-EN 1090-2 FOR STEEL WORK. MANUFACTURING IS ASSUMED TO BE DONE BY A BBC-CERTIFIED MANUFACTURER. ONLY MATERIAL THAT ARE CE-MARKED SHALL BE USED, CONCRETE AND REINFORCEMENT ARE EXCEPTIONS. THE SUPPLIER SHALL CARRY OUT AND BE RESPONSIBLE FOR DESIGN, MANUFACTURING, DELIVERY, ERECTION AND PERFORM REPAIR AND JOINT GROUTING OF PRECAST CONCRETE ELEMENTS. THE SUPPLIER IS RESPONSIBLE FOR ALL STRUCTURAL CALCULATIONS AND DESIGN OF SUPPLIED COMPONENTS, ATTACHMENTS ETC. AS WELL AS DRAWINGS AND REPORTS TO THE CLIENT AND THE AUTHORITIES. SHOP AND ASSEMBLY DRAWINGS SHALL BE DELIVERED TO THE CLIENT FOR REVIEW IN GOOD TIME BEFORE MANUFACTURING. THE SUPPLIER IS RESPONSIBLE FOR CO-ORDINATING JOINTS AND CONNECTIONS THAT AFFECT THE STEEL SUPPLIER, THE ROOF METAL SHEET SUPPLIER AND THE STRUCTURAL DESIGNER.

STANDARD TO WHICH EACH ELEMENT TYPE SHALL BE MANUFACTURED SS-EN 1168 HOLLOW CORE SLABS SS-EN 14843 STAIRS SS-EN 14992 WALL ELEMENTS

JOINTS IN PRECAST WALL ELEMENTS

WALL ELEMENTS WITH RADIATION SHIELDING REQUIREMENTS SHALL BE MANUFACTURED AND MOUNTED WITH AN OVERLAP IN THE JOINTS, SEE PRINCIPLE FIGURE BELOW. JOINTS SHALL BE FILLED WITH MORTAR.



PRECAST STAIRS SPECIFIC

CONCRETE STAIRS (INCLUDING WAIST AND LANDING SLABS) ACCORDING TO THE ARCHITECT DRAWINGS SHALL BE PERFORMED PRECAST AND WITH COATING ACCORDING TO THE ARCHITECT. STAIRS AND LANDINGS MUST BE SEPARATED FROM THE BUILDING STRUCTURE WITH HIDDEN IMPACT SOUND REDUCTION.

EXECUTION CLASS ACCORDING TO SS-EN 13670 EXECUTION CLASS 2

TOLERANCES FOR PRECAST CONCRETE

TOLERANCES ACCORDING TO VALID PRODUCT STANDARD, CLASS B TABLE GSC/1 IN HUS-AMA AND AND SS-EN 13670 TOLERANCE CLASS 2. REGARDING IN-CAST DETAILS AND CONCRETE ELEMENTS WHERE STEEL STRUCTURES CONNECT SS-EN 1090-2 CLASS 1 SHALL APPLY IF NOTHING ELSE I STATED.

SCALE 1:100 A1

TOLERANCES FOR IN SITU CASTED CONCRETE

ACCORDING TO SS-EN 13670. TOLERANCE CLASS 1 ARE VALID FOR IN-SITU CAST CONCRETE.

VALID TOLERANCES ACCORDING TO SS-EN 1090-2 CLASS 1 SHALL BE APPLIED TO ATTACHED INSERTS AND CONCRETE TO WHICH STEEL STRUCTURES SHALL BE CONNECTED UNLESS OTHERWISE STATED.

ACCORDING TO SS-EN 13670, EXECUTION CLASS 2.

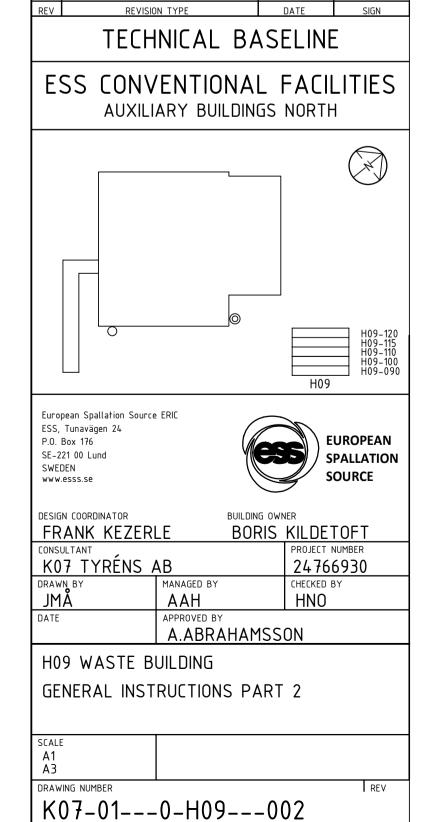
CONCRETE FLOORS TYPE G01 AND G02

JOINTS IN CONCRETE FLOORS ARE DONE AS A FORMED FREE-MOVEMENT JOINT USING SYSTEM ALPHAJOINT OR A COMPARABLE SYSTEM.

DESIGN AND EXECUTION SHALL BE DONE IN ACCORDANCE WITH GUIDELINES IN TECHNICAL REPORT 34 - CONCRETE INDUSTRIAL GROUND FLOORS, CONCRETE SOCIETY, 2014.

TOLERANCE DEMANDS

TOLERANCES FOR CONCRETE FLOORS, TYPE G01 AND G02, ACCORDING TO CLASS A, TABLE GSC/1 IN HUS AMA



C1-ENVIROMENT: INTERIOR STEEL STRUCTURES. C2-ENVIROMENT: STEEL STRUCTURES BUILT INTO EXTERNAL WALLS. C3-ENVIROMENT:

C4-ENVIROMENT: EXTERIOR STEEL STRUCTURES. IM3-ENVIROMENT: STEEL STRUCTURES IMMERSED IN SOIL.

PAINT SYSTEM

ACCORDING TO SS-EN ISO 12944-5 AND "HANDBOK FÖR TILLÄMPNING AV SS-EN 1090-2" WITH EXPECTED DURABILITY AS TABLE BELOW.

C1-ENVIROMENT: C2 LOW C2-ENVIROMENT: C2 HIGH C3-ENVIROMENT: C3 HIGH C4-ENVIROMENT: C4 HIGH IM3-ENVIROMENT: Im3 HIGH

ALL PAINT WORK SHALL BE DONE IN COMPLIANCE TO SS-EN 1090-2 CHAPTER 10, SS-EN 12944-7, PAINT MANUFACTURERS DESCRIPTION AND "HANDBOK I ROSTSKYDDSMÅLNING" FROM SWEREA/KIMAB.

PAINT WORKS ALSO NEED TO FULFILL THE REQUIREMENTS STATED IN PAKT/TBY - "TECHNICAL REGULATIONS FOR SURFACE TREATMENT"

ON-SITE WELD ASSEMBLIES SHALL BE LEFT UNPAINTED 150MM FROM WELD IF WELDING IS TO BE DONE AFTER PROTECTIVE PAINTING. AFTER WELDING, STEEL SHALL BE PAINTED WITH THE SAME PAINT SYSTEM AS THE REST OF THE ASSEMBLY.

PREPARATION OF STEEL SUBSTRATES

ALL SURFACES SHALL BEFORE BLASTING BE FREE FROM OIL, SALTS AND OTHER POLLUTION ACCORDING TO SS-EN ISO 12944-4 6.1.3 AND 6.1.4. IF NOT STATED OTHERWISE THE SURFACE SHALL BE BLASTED TO A MINIMUM VALUE OF Sa 2½ ACCORDING TO SS-EN 8501-1. PREPARATION GRADES OF WELDS, CUT EDGES AND OTHER AREAS WITH SURFACE IMPERFECTIONS SHALL CONFORM TO CHOSEN PAINT SYSTEM DURABILITY ACCORDING TO SS-EN 1090-2 CHAPTER 10.2 AND HUS-AMA GSM-1. FOR Im3 HIGH, PREPARATION GRADE P3 SHALL APPLY.

THE STEEL CONTRACTORS WORK SHOP(S) SHALL EACH BE CERTIFIED, BY A BODY ADEQUATELY ACCREDITED, ACCORDING TO EN 1090-1:2009+A1:2011 FOR THE EQUIVALENT EXECUTION CLASS (EXC2 OR EXC3 FOR CRANE BEAMS). THE METHOD OF CERTIFICATION SHALL MATCH THE TENDER PROCEDURE. THE WELDING COMPANY SHALL BE CERTIFIED ACCORDING TO SS-EN ISO 3834-2 OR -3 (TO THE EQUIVALENT EXECUTION CLASS) TO PERFORM THE WELDS ON SITE. MANUFACTURING AND ASSEMBLY OF STEEL STRUCTURES SHALL BE SUPERVISED BY THE PERSON IN CHARGE WHO SHALL HAVE PROVEN COMPETENCE TR-STÅL/N (OR TR-STÅL/K IN EXC3) ACCORDING TO AMA, OR CORRESPONDING INTERNATIONAL SKILL OR COMPETENCE. THE PERSON IN CHARGE OF WELDING SHALL HAVE COMPETENCE ACCORDING TO THE WELDING COMMISSION GUIDELINES -TILLSYN VID SVETSNING- AND ACCORDING TO SS-EN ISO 17660-1 IF WELDING IN REINFORCEMENT IS PERFORMED. ALL WELDING PERSONAL SHALL HAVE PROVEN COMPETENCE ACCORDING TO SS-EN ISO 9606-1. ALL THE PERSONS WELDING IN REINFORCEMENT SHALL ALSO FULFILL THE DEMANDS IN SS-EN 17660-1.

MATERIALS

SECTIONS HEA/HEB/	IPE:	S355J2	SS-EN 10025-2
SECTIONS UPE:		S355N	SS-EN 10025-3
SECTIONS L:		S235JR	SS-EN 10025-2
HOLLOW SECTIONS V	/KR/VCKR:	S355J2H	SS-EN 10210
HOLLOW SECTIONS K	KR/KCKR:	S355J2H	SS-EN 10219
SHEETS AND PLATE	S:	S355J2	SS-EN 10025-2
ANCHOR BOLTS:		S355J2	SS-EN 10025-2
BOLTED NON-PRELO	ADED		
CONNECTIONS:	BOLT	M6S 8.8 †Zn	SS-EN ISO 4014, 4017
	NUT	M6M 8 tZn	SS-EN ISO 4032
	WASHER	200 HV †Zn	SS-EN ISO 7090

COLD FORMED STRUCTURAL HOLLOW SECTIONS ACCORDING TO SS-EN 10219 SHALL TOGETHER WITH A THICKNESS NOT EXCEEDING 12.5 mm AND ARE AL-KILLED (AL \geq 0,20%) AND SATISFY C \leq 0.18%, P \leq 0,020%, S \leq 0,012%.

FASTENERS INTENDED FOR NON-PRELOADED BOLTED CONNECTIONS SHALL BE CE MARKED ACCORDING TO SS-EN 15048-1 AND DELIVERED WITH A CERTIFICATE OF INSPECTION 3.1 ACCORDING TO SS-EN 10204. GALVANIZING OF FASTENERS SHALL BE PERFORMED ACCORDING TO SS-EN ISO 10684.

WELDING PLAN

WELDING PLAN TO BE CARRIED OUT BY THE STEEL CONTRACTOR

EXECUTION

EXECUTION CLASS, CRANE BEAMS: EXC3 EXECUTION CLASS, OTHER STRUCTURES: EXC2 THERMAL CUTTING: RANGE 4 WELD QUALITY LEVEL, CRANE BEAMS: WELD QUALITY LEVEL, OTHER STRUCTURES: C WITH ADDITIONS AND

EXCEPTIONS ACCORDING TO SS-EN 1090-2.7.6.

CATEGORIES OF BOLTED NON - PRELOADED

PREPARATION GRADE P2.

CONNECTIONS: A AND D SURFACES TO BE PROTECTED WITH CORROSION PROTECTION SYSTEMS IN COMPLIANCE WITH CORROSIVITY CATEGORY C3 OR HIGHER (C4, IM3) SHALL BE EXECUTED ACCORDING TO SS-EN ISO 8501-3 PREPARATION GRADE P3. OTHER SURFACES SHALL BE EXECUTED ACCORDING TO SS-EN ISO 8501-3

ERECTION SCHEDULE

ERECTION SCHEDULE COVERING E.G. TEMPORARY BRACING AND STABILIZING SHALL BE ESTABLISHED BY THE CONTRACTOR IN AGREEMENT WITH THE STRUCTURAL DESIGN ENGINEER, SEE ALSO UNDER STABILITY ON DRAWING -001.

FIRE PROTECTION

FIRE PROTECTION ACCORDING TO THE FIRE SAFETY STRATEGY REPORT.

ALL STRUCTURES SUPPORTING EI60 FIRE COMPARTMENTS FIRE PROTECTION OF STEEL STRUCTURES IS DONE USING COATING ON VISIBLE STEEL AND FIRE PROTECTION BOARDS ON BUILT-IN STEEL TO THE APPROPRIATE CLASS.

ESSENTIAL TOLERANCES ACCORDING TO SS-EN 1090-2 ANNEX D. FUNCTIONAL TOLERANCES IF NOTHING ELSE IS STATED ACCORDING TO SS-EN 1090-2 ANNEX D CLASS 1.

FUNCTIONAL TOLERANCES FOR CRANE BEAMS ACCORDING TO SS-EN 1090-2 ANNEX D CLASS 2.

INSPECTION OF EXECUTION

INSPECTION SHALL BE CARRIED OUT ACCORDING TO SS-EN 1090-2 AND SBI'S MANUAL FOR APPLICATION OF SS-EN 1090-2. ADDITIONAL INSPECTIONS AND CONTROLS ACCORDING TO SEPARATE INSPECTION PLAN MADE BY THE STRUCTURAL ENGINEER COORDINATED WITH THE STEEL CONTRACTOR.

ROOF TRUSSES AND OTHER STEEL STRUCTURES

ROOF TRUSSES INCLUDING SUPPORT DETAILS SHALL BE DESIGNED BY THE MANUFACTURER WITH ACTUAL DEAD LOADS FROM THE ROOF STRUCTURE, SNOW AND WIND LOADS WITH RELEVANT SHAPE COEFFICIENTS AND WITH INSTALLATION LOADS. THE EFFECTS OF THE CONTINUITY OF THE LOAD BEARING METAL SHEET SHALL BE TAKEN INTO ACCOUNT WITH A CONTINUITY FACTOR OF 1.10.

THE ROOF TRUSSES SHALL BE MANUFACTURED WITH A CAMBER EQUIVALENT TO THE DEFLECTION FROM PERMANENT LOAD.

TRUSSES ARE TO BE DESIGNED WITH LATERAL AND LATERAL TORSIONAL BRACING OF THE TOP CHORD USING THE ROOF METAL SHEET

SHOP AND ASSEMBLY DRAWINGS SHALL BE DELIVERED TO THE CLIENT FOR REVIEW IN GOOD TIME (IN AGREEMENT WITH THE CLIENT) BEFORE MANUFACTURING.

LOAD-BEARING SHEET METAL STRUCTURES

ROOF SHEET METAL INCLUDING FIXING, EDGE FLASHING, ANCHORS, CONNECTIONS AND ALL PARTS NEEDED FOR INTENDED FUNCTION, SHALL BE DESIGNED BY THE SUPPLIER WITH THE ACTUAL DEAD LOAD FROM THE ROOF STRUCTURE, SNOW AND WIND LOADS WITH RELEVANT SHAPE COEFFICIENTS AND WITH INSTALLATION LOADS.

THE METAL SHEET SHALL BE DESIGNED TO PREVENT BOTH LATERAL AND LATERAL TORSIONAL BUCKLING OF THE STEEL TRUSS BY CONNECTION THE METAL SHEET TO THE TOP CHORD. STRUCTURAL SYSTEM AND JOINTS FOR THE LOAD BEARING METAL SHEET

SHALL BE DONE ENSURING A CONTINUTY FACTOR NOT EXCEEDING 1,10. THE ROOF PANEL IS ALSO USED AS A PART OF THE STABILIZING OF THE BUILDING. THE METAL SHEET IS A LARGE SHEET WITH THE FUNCTION TO DISTRIBUTE HORIZONTAL LOADS TO VERTICAL STIFFENERS. THIS SHALL BE TAKEN INTO ACCOUNT, (STRESSED SKIN DESIGN). THE SHEET METAL SHALL BE DESIGNED IN THE SAME SAFETY CLASS AS THE SUPPORTING STRUCTURE ITSELF FOR ALL LOAD COMBINATIONS, COMPARE SAFETY CLASS ON DRAWING -001. SHOP AND ASSEMBLY DRAWINGS SHALL BE DELIVERED TO THE CLIENT FOR REVIEW IN GOOD TIME (IN AGREEMENT WITH THE CLIENT) BEFORE MANUFACTURING.

INSPECTION PLAN

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BOLTING ASSEMBLIES AND WELDS

BOLTING ASSEMBLIES

BOLTING ASSEMBLIES FOR NON PRELOADED APPLICATION SHALL BE CE-MARKED ACCORDING TO SS-EN 15048-1. INSPECTION CERTIFICATE 3.1 ACCORDING TO SS-EN 10204 SHALL BE INCLUDED WITH THE DELIVERY. HOT DIP GALVAINZATION SHALL CONFORM TO TO SS-EN ISO 10684.

HEX HEAD SCREW FULLY THREADED SS-EN ISO 4017 8.8 tZn HEX HEAD SCREW PARTLY THREADED SS-EN ISO 4014 8.8 tZn WASHER SS-EN ISO 7090 300 HV tZn SS-EN ISO 4032 8 tZn HEXAGON NUT

WASHERS SHALL BE USED IN ALL BOLTING ASSEMBLIES. CONNECTIONS SHALL BE IN FULL CONTACT BEARING. BOLTING ASSEMBLIES SHALL BE LOCKES WITH HEAVY PUNCH MARK OR LOCK NUT.

<u>WELDS</u>

THE FILLER METAL SHALL BE MATCHING, MEANING THAT YIELD STRENGTH, ULTIMATE TENSILE STRENGTH, ELONGATION AT FAILURE AND MINIMUM CHARPY V-NOTCH ENERGY VALUE OF THE FILLER METAL, SHOULD BE EQUIVALENT TO, OR BETTER THAN THAT SPECIFIED FOR THE PARENT MATERIAL.

FACING SURFACES BETWEEN PROFILE AND BASE/TOP PLATE SHOULD BE WELL FITTED TO EACH OTHER AND BE IN FULL CONTACT. FACING SURFACES BETWEEN STIFFENER AND FLANGE SHOULD BE WELL FITTED TO EACH OTHER AND BE IN FULL CONTACT.

FOR WELDING SYMBOLS IS SS-EN ISO 2553 SYSTEM A USED. ALL WELDS ARE OR UNLESS OTHERWISE STATED.

EQUIPOTENTIAL BONDING

ALL COLUMNS ARE DONE WITH A UNPAINTED HEX HEAD FULLY SCREW M10 WELDED TO THE COLUMN 300MM FROM THE BASE ACCORDING TO PRINCIPLE DETAIL BELOW.

SECONDARY STEEL STRUCTURES

ALL SECONDARY STEEL STRUCTURES NEEDS TO BE COORDINATED WITH ANY CONTRACTOR THAT ARE IN NEED OF THE SECONDARY STEEL. FOR EXAMPLE GATE STEEL FRAMES, FACADE FIXING.

7 GEOTECHNICAL STRUCTURE

GEOTECHNICHAL INVESTIGATIONS

GEOTECHNICAL INVESTIGATIONS ARE CARRIED OUT AND DOCUMENTED IN ESS-0005223, G02-DT-TBSIGD----GDR, dated 2016-02-26 ESS, 0005205, G02-DT-TBSIGD----GIR, dated 2016-02-26.

FROSTLESS DEPTH IS SET TO 1,3 m BELOW GROUND SURFACE.

CHARACTERISTIC VALUE OF GROUND WATER LEVEL IS NOT YET DECIDED IN THIS STAGE.

THE CONTRACTOR SHALL PERFORM AND BE LIABLE OF DRAINAGE AND GROUNDWATER LOWERING ALONG BUILDINGS IN THE EXECUTION PHASE IF

LOWERING OF STATIC PORE-WATER PRESSURE MUST BE MADE IN THE EXCAVATION BY DITCHES WITH DRAINAGE AND PUMP PITS ARRANGED SO THE PRESSURE LEVEL IN THE CLAY TILL IS LOWERED AT LEAST 0,5 m BELOW LOWEST FOUNDATION LEVEL AND TO LET RAINWATER BE

EQUIPMENT FOR GROUNDWATER LOWERING SHALL BE INSTALLED SO THE EXCAVATION BOTTOM BELOW FUTURE FOUNDATIONS NOT WILL BE DISTURBED.

EXCAVATION

EXCAVATION CAN TAKE PLACE WITH UNLOADED SLOPE IN GRADIENT 2:1 AND WITH BOTTOM OF THE SLOPE ABOUT 1,5 m OUTSIDE THE STRUCTURE (FREE EXCAVATION).

UNCOVERED BOTTOM OF EXCAVATIONS MUST NOT BE EXPOSED TO RAIN DUE TO SENSIVITY OF WATER INFLUENCE. EXCAVATION FOR FOUNDATIONS SHALL BE CARRIED OUT PURSUANT TO THE FOLLOWING PROCEDURE. 1. GROUNDWATER LEVEL LOWERED BY DITCHES WITH DRAINAGE AND

- PUMP PITS ARRANGED SO THE PRESSURE LEVEL IN THE CLAY TILL IS LOWERED AT LEAST 0,5 m BELOW LOWEST FOUNDATION LEVEL.
- 2. THE FINAL EXCAVATION SHALL BE CARRIED OUT AT DRY WEATHER AND BE DONE WITH BUCKET WITHOUT TEETH.
- 3. THE BOTTOM OF EXCAVATIONS SHALL BE INSPECTED AND APPROVED BY THE ENGINEER ACCORDING TO THE PLAN OF SUPERVISION.
- 4. IMMEDIATELY AFTER UNCOVERING AND INSPECTION THE EXCAVATION BOTTOM SHALL BE PROTECTED WITH COARSE CONCRETE OR GRAVEL.

FOUNDATIONS SHALL BE DESIGNED IN GEOTECHNICAL CLASS 2 (GK2) AND SECURITY CLASS 2 (SK 2) ACCORDING TO THE GEOTECHNICAL DESIGN

SOME FOUNDATIONS SHALL BE PERFORMED ON COMPACTED AND STABILIZED FILLING ON TOP OF UNDISTURBED CLAY TILL.

DESIGN RESISTANCE IN BOTH ULTIMATE AND SERVICEABILITY LIMIT STATES ARE TO BE CALCULATED ACCORDING TO SS-EN 1997-1 WITH PARAMETERS AND PARTIAL FACTORS FROM THE GEOTECHNICAL DESIGN REPORT GDR.

- FOR DESIGN CONCERNING BEARING RESISTANCE IN CLAY TILL, CALCULATIONS ARE TO BE PERFORMED FOR BOTH DRAINED AND NON-DRAINED PARAMETERS, THE MOST UNFAVORABLE CASE DEFINES

CAPILLARY BREAKING AND DRAINAGE LAYER BELOW SLAB ON GROUND CONSIST OF MINIMUM 250 mm CRUSHED AGGREGATE. THE DRAINING LAYER IS TO BE CONNECTED TO THE EXTERIOR DRAINAGE PIPES AROUND THE BUILDING.

DRAINAGE PIPE:

THE DRAINAGE PIPES CONSISTS OF DRAIN LINES Ø110 PLASTIC PIPES ACCORDING TO AMA ANLÄGGNING 10 PB-.53. THE PIPES SHALL BE IN STRAIGHT RIGID LENGTHS, COMPLETE WITH BENDS, JOINT SLEEVES, RINSING WELLS ETC. THE PIPES ARE TO BE PLACED ON 50 mm MACADAM AND BACKFILLED ON REMAINING SIDES WITH A MINIMUM OF 100 mm MACADAM AND LAID WITH A GRADIENT OF MINIMUM 1:200 TOWARDS DRAINAGE WELLS. A NONWOVEN GEOTEXTILE SHALL BE PLACED AROUND THE BACKFILL.

BACKFILLING BACKFILLING SHOULD BE DONE EITHER WITH LIME STABILIZED MATERIAL

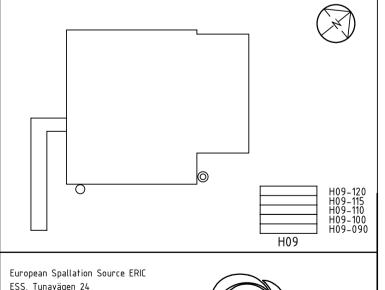
INSPECTION

ACCORDING TO GROUND EXCAVATION OR 0-90 CRUSHED MATERIAL IF STABILIZING IS NOT NECESSARY.

INSPECTIONS ARE TO BE CARRIED OUT ACCORDING SS-EN 1997-1.

PD DRAFT 2018-01-09

REVISION TYPE DATE SIGN TECHNICAL BASELINE ESS CONVENTIONAL FACILITIES AUXILIARY BUILDINGS NORTH



ESS. Tunavägen 24 P.O. Box 176 SE-221 00 Lund SWEDEN

DESIGN COORDINATOR

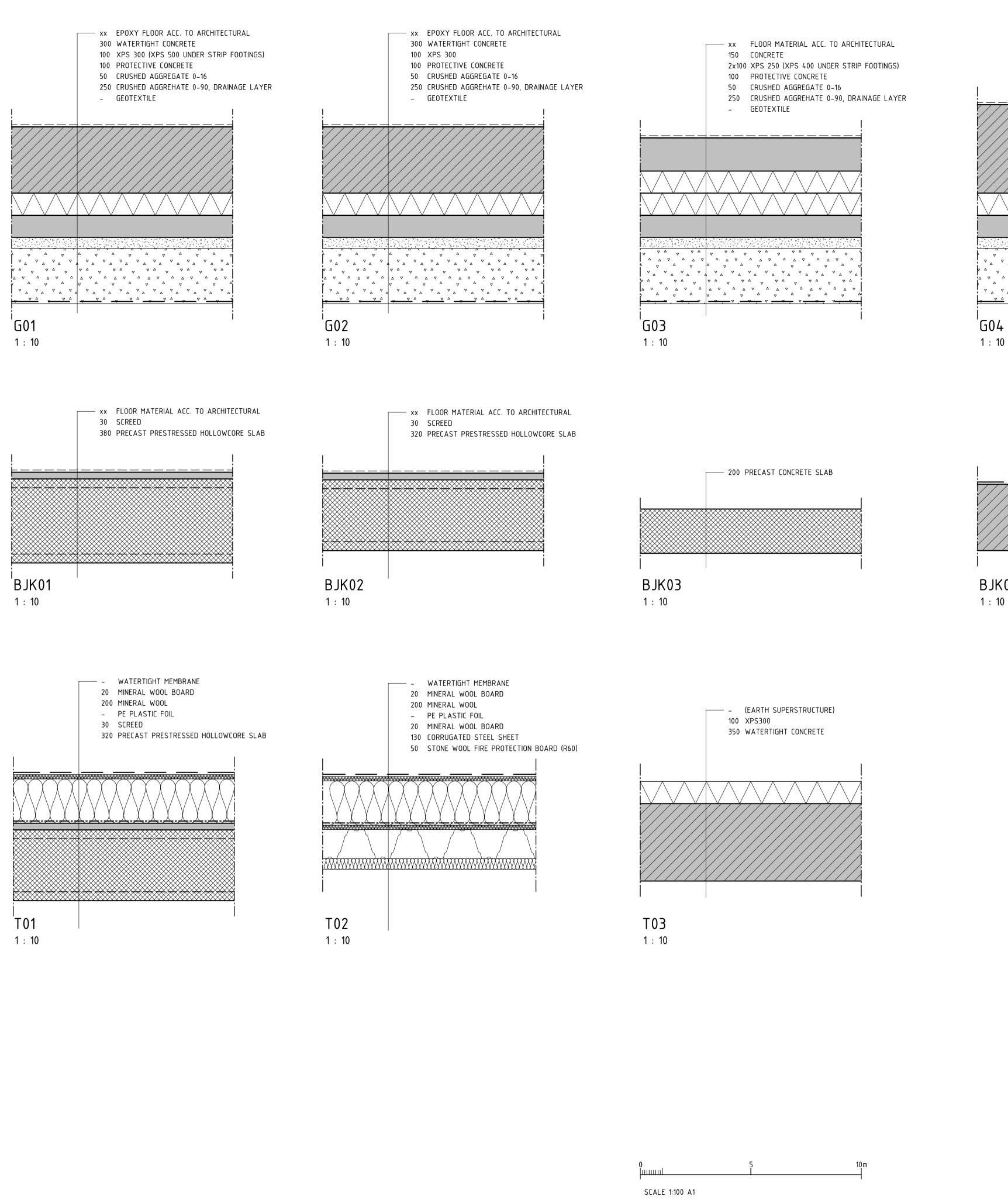
EUROPEAN ess **SPALLATION** SOURCE BUILDING OWNER BORIS KILDETOFT

FRANK KEZERLE K07 TYRÉNS AB 24766930 MANAGED BY CHECKED BY AAHHNO A.ABRAHAMSSON

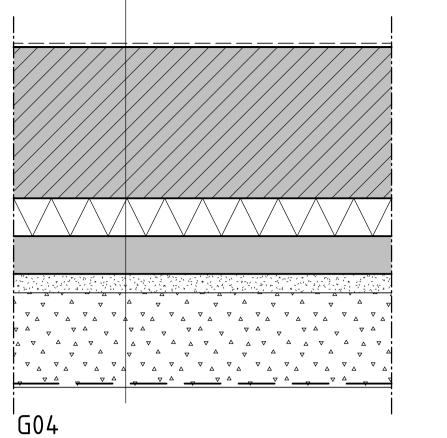
H09 WASTE BUILDING GENERAL INSTRUCTIONS PART 3

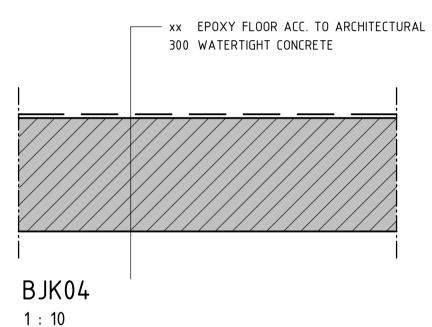
K07-01---0-H09---003

SCALE 1:100 A1



xx EPOXY FLOOR ACC. TO ARCHITECTURAL
400 WATERTIGHT CONCRETE
100 XPS 300 (XPS 500 UNDER STRIP FOOTINGS)
100 PROTECTIVE CONCRETE
50 CRUSHED AGGREGATE 0-16
250 CRUSHED AGGREHATE 0-90, DRAINAGE LAYER
- GEOTEXTILE





PD DRAFT 2018-01-09

REFERENCES

SEE PLAN DRAWINGS

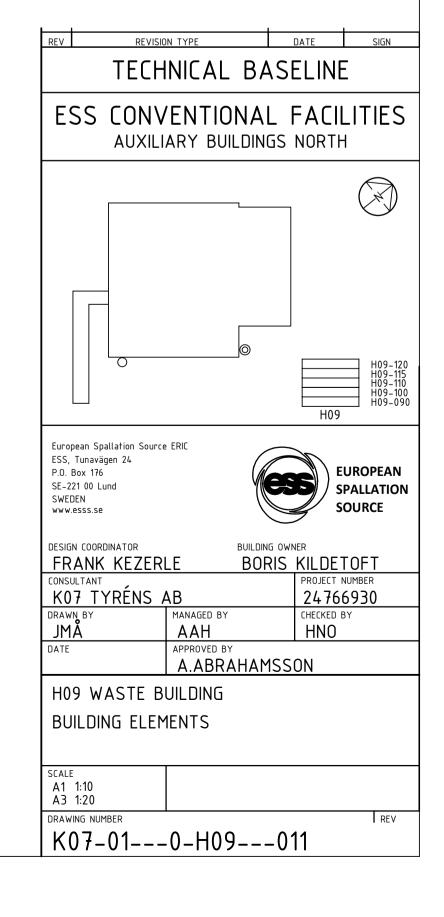
GENERAL INSTRUCTIONS ACCORDING

TO K07-01---0-H09---001--003

BUILDING ELEMENTS ACCORDING

TO K07-01---0-H09---011-012

LOCATION OF BUILDING ELEMENTS,

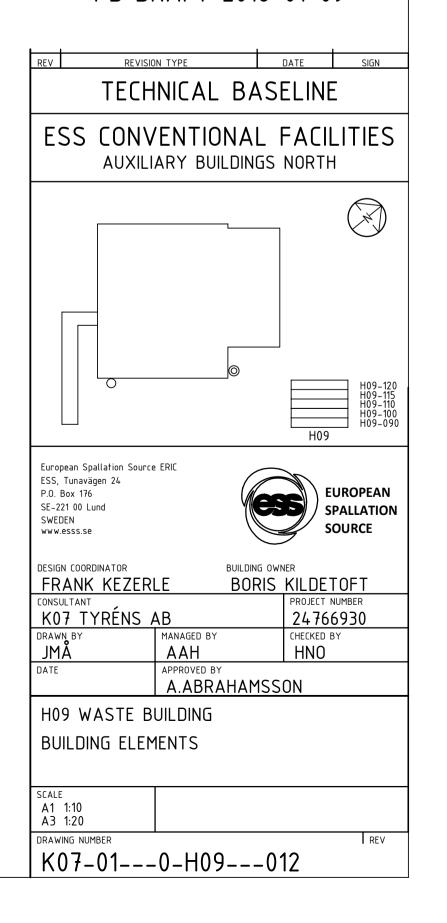


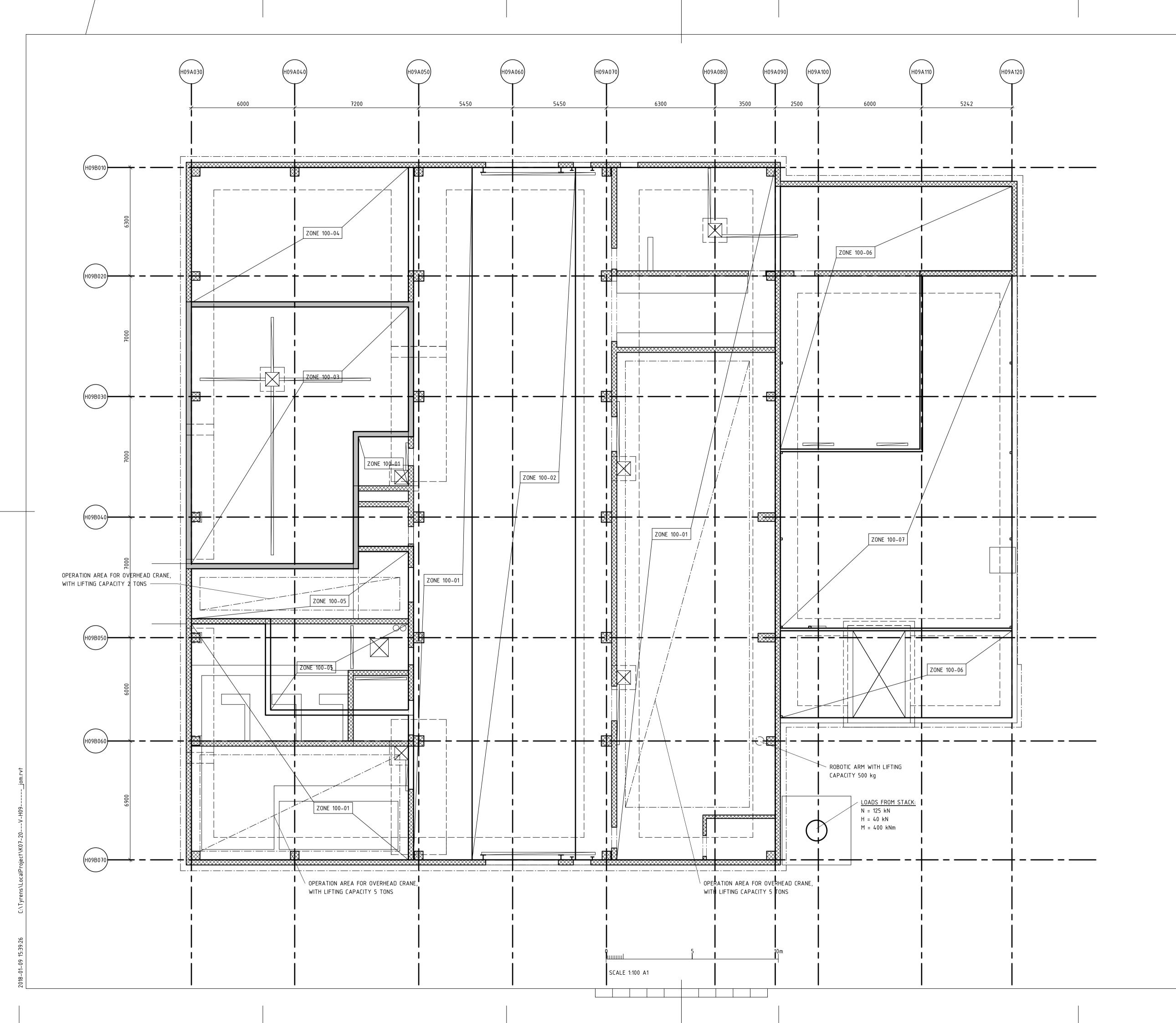
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GENERAL INSTRUCTIONS ACCORDING TO K07-01---0-H09---001--003

BUILDING ELEMENTS ACCORDING TO K07-01---0-H09---011-012

LOCATION OF BUILDING ELEMENTS, SEE PLAN DRAWINGS





GENERAL INSTRUCTIONS ACC.TO K07-01---0-H09---001--003 BUILDING ELEMENTS ACC. TO K07-01---0-H09---011-012

EXPLANATIONS

ALL LOADS ARE GIVEN AS CHARACTERISTIC VALUES IF NOT STATED OTHERWISE.

LIVE LOADS TABLE BELOW SHOWS MAXIMUM ALLOWED UNIFORMLY DISTRIBUTED OR MAXIMUM ALLOWED

CONCENTRATED LOAD. LOAD REGULATION IS THAT THE UNIFORMLY DISTRIBUTED

LOAD AND THE CONCENTRATED LOAD MUST NOT OCCUR

SIMOULTANEOUSLY. SEE ALSO GENERAL INSTRUCTIONS DRAWINGS FOR FURTHER LOAD CONDITIONS.

LOADS

DEAD LOADS

SEE K07-01---0-H09-001 AND CORRESPONDING PLAN DRAWINGS.

LIVE LOADS

						
ZONE	CATEGORY	q_k	Q_k	Ψ_0	Ψ_1	Ψ
		kN/m²	kN			
100-01	E2	50	60	1,0	0,9	0,
100-02	E2	50	60	1,0	0,9	0,
100-03	E2	70	350	1,0	0,9	0,
100-04	E2	150	300	1,0	0,9	0,
100-05	E2	40	50	1,0	0,9	0,
100-06	E2	5	5	1,0	0,9	0,
100-07	В	2,5	3	0,7	0,5	0
110-01	E2	15	50	1,0	0,9	0,
110-02	E2	15	50	1,0	0,9	0
110-03	E2	10	80	1,0	0,9	0,
110 – 11	Н	0,4	1	0	0	(
115-01	E2	5	5	1,0	0,9	0,
120-11	Н	0,4	1	0	0	(
		•				

SNOW LOAD

SEE K07-01---0-H09-001.

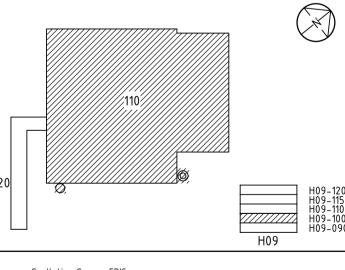
SNOW LOAD OCCUR ON ZONES 110-11 AND 120-11. FOR ZONE 110-11, SNOW CONCENTRATIONS SHALL BE CONSIDERED.

LOADS FROM ATB 12K OCCUR IN ZONE 100-02. LOADS FROM ATB 12K SHALL BE COMBINATED WITH LIVE LOADS AND LOADS FROM FORKLIFT.

LOADS FROM FORKLIFT CAN OCCUR IN ANY POINT IN ZONE 100-01, 100-02 AND 100-04. LOADS FROM FORKLIFT SHALL BE COMBINATED WITH LIVE LOADS AND LOADS FROM ATB 12K.

EXCEPTIONAL INTERNAL WATER PRESSURE WATER PRESSURE ON WALLS IN CASE OF INTERNAL FLOODING SHALL BEPPRISITER FOR TOND 1983. 01.09
ASSUMED MAXIMUM WATER LEVEL: 1500mm ABOVE FFL.

REV	REVISION TYPE	DATE	SIGN
	TECHNICAL BA	SELINE	-
E:	SS CONVENTIONA	L FACII	LITIES



European Spallation Source ERIC ESS, Tunavägen 24 P.O. Box 176 SE-221 00 Lund SWEDEN

EUROPEAN SPALLATION SOURCE

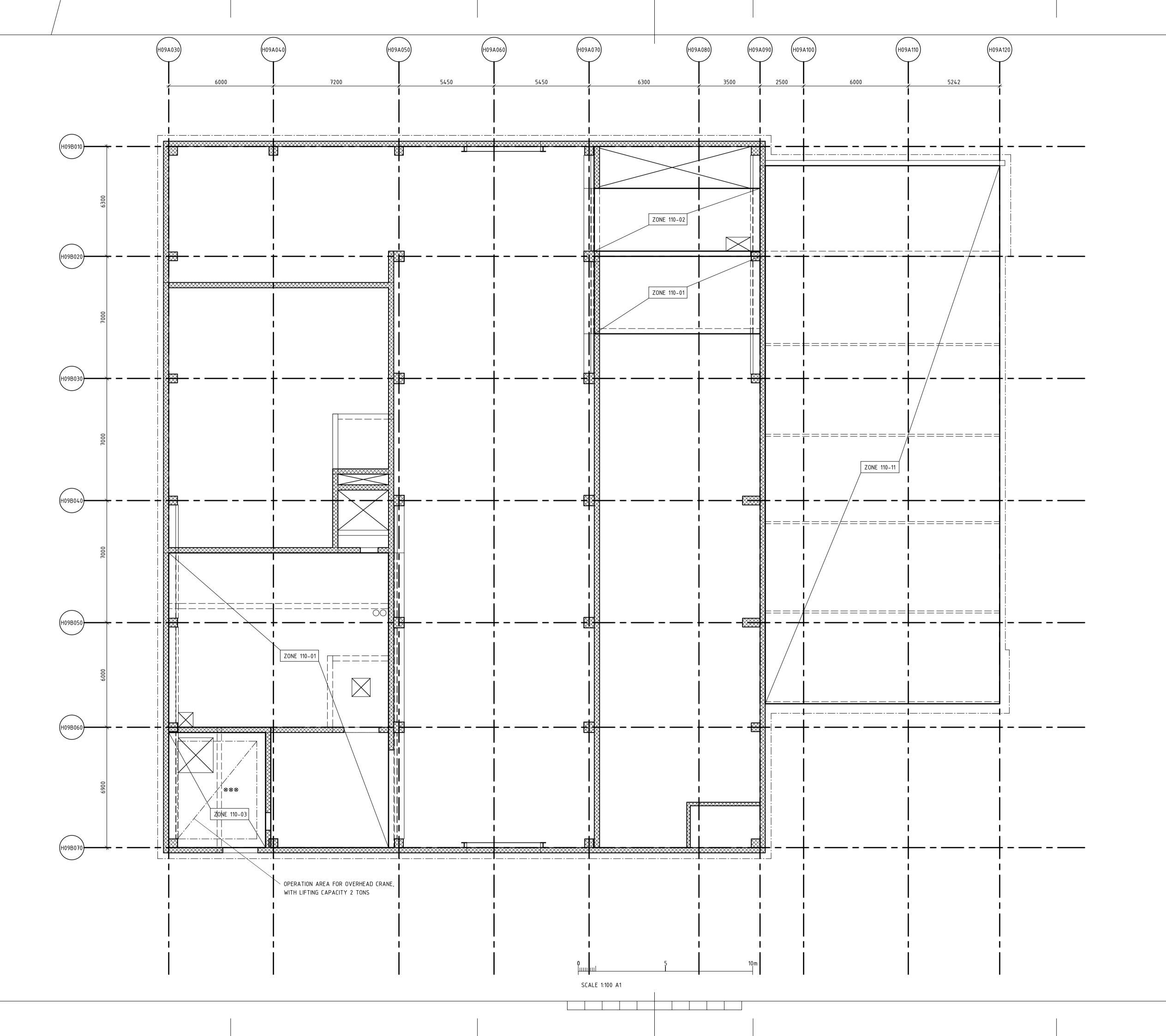
DESIGN COORDINATOR BUILDING OWNER BORIS KILDETOFT FRANK KEZERLE CONSULTANT K07 TYRÉNS AB 24766930 AAH HNO APPROVED BY

A.ABRAHAMSSON

H09 WASTE BUILDING

LOAD PLAN, LEVEL H09-100, ZONE 110

SCALE A1 1:100 A3 1:200



GENERAL INSTRUCTIONS ACC.TO K07-01---0-H09---001--003
BUILDING ELEMENTS ACC. TO K07-01---0-H09---011-012

EXPLANATIONS

ALL LOADS ARE GIVEN AS CHARACTERISTIC VALUES IF NOT STATED OTHERWISE.

LIVE LOADS TABLE BELOW SHOWS MAXIMUM ALLOWED UNIFORMLY DISTRIBUTED <u>OR</u> MAXIMUM ALLOWED CONCENTRATED LOAD.

LOAD REGULATION IS THAT THE UNIFORMLY DISTRIBUTED LOAD AND THE CONCENTRATED LOAD MUST NOT OCCUR SIMOULTANEOUSLY.

SEE ALSO GENERAL INSTRUCTIONS DRAWINGS FOR FURTHER LOAD CONDITIONS.

LOADS

DEAD LOADS

SEE K07-01---0-H09-001 AND CORRESPONDING PLAN DRAWINGS.

LIVE LOAI	<u>DS</u>				
ZONE	CATEGORY	q _k kN/m²	Q _k kN	Ψ_0	Ψ_1
100-01	E2	50	60	1,0	0,9
100-02	E2	50	60	1,0	0,9
100-03	E2	70	350	1,0	0,9
100-04	E2	150	300	1,0	0,9
100-05	E2	40	50	1,0	0,9
100-06	E2	5	5	1,0	0,9

10-01 E2 15 50 1,0 0,9 0,8 10-02 E2 15 50 1,0 0,9 0,8 10-03 E2 10 80 1,0 0,9 0,8 10-11 H 0,4 1 0 0 0 0 15-01 E2 5 5 1,0 0,9 0,8

2,5 3 0,7 0,5 0,3

120-11 H 0,4 1 0 0 0 0 SNOW LOAD

SEE K07-01---0-H09-001.

SNOW LOAD OCCUR ON ZONES 110-11 AND 120-11. FOR ZONE 110-11, SNOW CONCENTRATIONS SHALL BE CONSIDERED.

ATB 12K:

LOADS FROM ATB 12K OCCUR IN ZONE 100-02. LOADS FROM ATB 12K SHALL BE COMBINATED WITH LIVE LOADS AND LOADS FROM FORKLIFT.

<u>FORKLIFT</u>

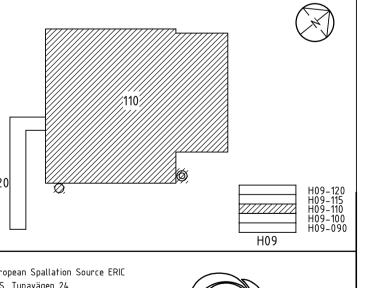
LOADS FROM FORKLIFT CAN OCCUR IN ANY POINT IN ZONE 100-01, 100-02 AND 100-04. LOADS FROM FORKLIFT SHALL BE COMBINATED WITH LIVE LOADS AND LOADS FROM ATB 12K.

EXCEPTIONAL INTERNAL WATER PRESSURE

WATER PRESSURE ON WALLS IN CASE OF INTERNAL FLOODING
SHALL BEPTINSITER OF 1983. 01.09
ASSUMED MAXIMUM WATER LEVEL: 1500mm ABOVE FFL.

REV	REVISION TYPE	DATE	SIGN
	TECHNICAL BA	ASFLINE	-
	recrimente bi	, o L L II 1 L	_

ESS CONVENTIONAL FACILITIES



European Spallation Source ERIC ESS, Tunavägen 24 P.O. Box 176 SE-221 00 Lund SWEDEN www.esss.se

EUROPEAN SPALLATION SOURCE

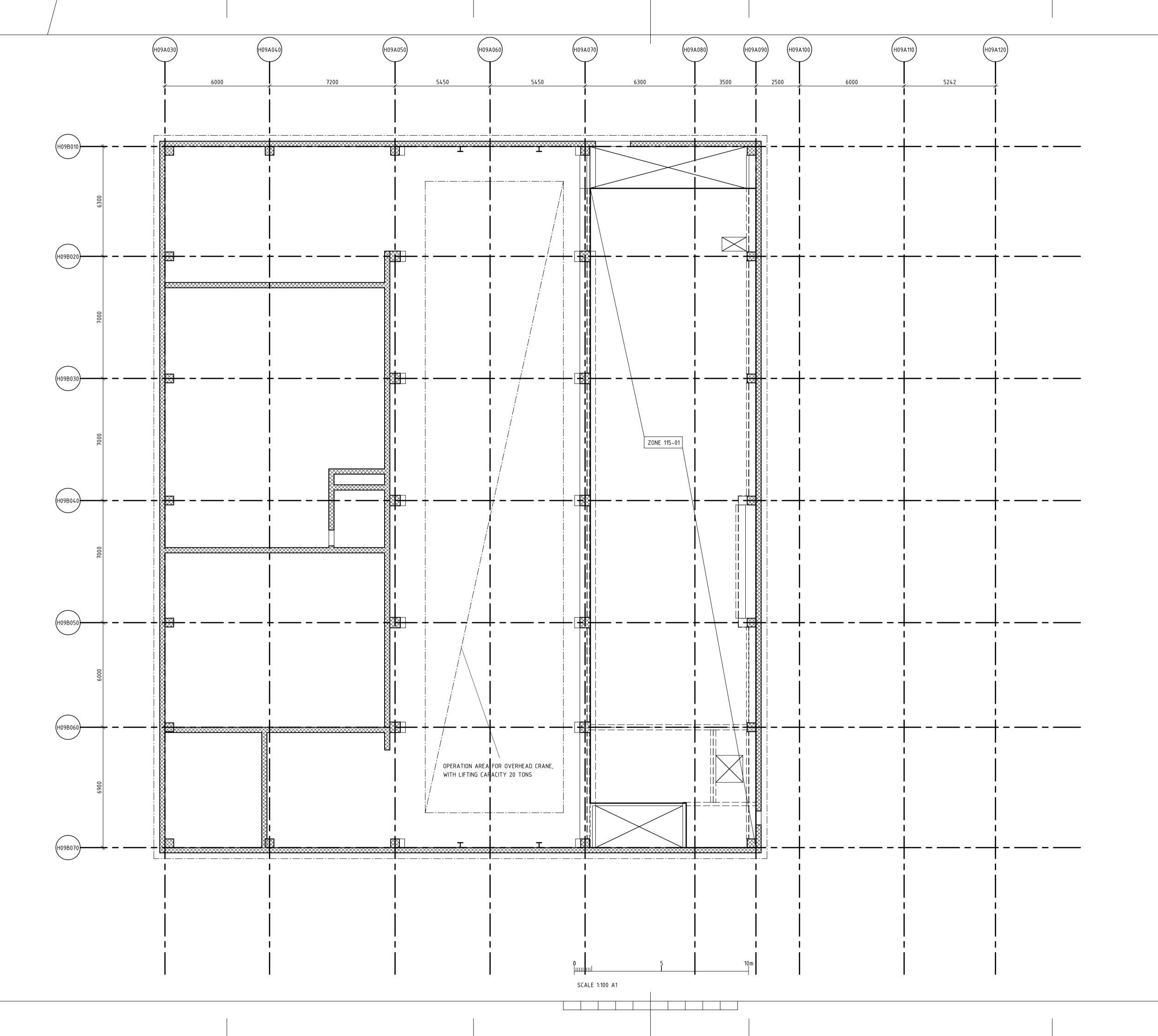
DESIGN COORDINATOR		BUILDING OWN	NER
FRANK KEZERI	LE	BORIS	KILDETOFT
CONSULTANT			PROJECT NUMBER
K07 TYRÉNS A	AB		24766930
DRAWN BY	MANAGED BY		CHECKED BY
JMÅ	AAH		HNO
DATE	APPROVED BY		
	Λ Λ RR Λ	PAMCS	∩N

H09 WASTE BUILDING

LOAD PLAN, LEVEL H09-110, ZONE 110

SCALE
A1 1:100
A3 1:200

DRAWING NUMBER



GENERAL INSTRUCTIONS ACC.TO K07-01---0-H09---001--003 BUILDING ELEMENTS ACC. TO K07-01---0-H09---011-012

EXPLANATIONS

ALL LOADS ARE GIVEN AS CHARACTERISTIC VALUES IF NOT STATED OTHERWISE.

LIVE LOADS TABLE BELOW SHOWS MAXIMUM ALLOWED UNIFORMLY DISTRIBUTED <u>OR</u> MAXIMUM ALLOWED

CONCENTRATED LOAD. LOAD REGULATION IS THAT THE UNIFORMLY DISTRIBUTED LOAD AND THE CONCENTRATED LOAD MUST NOT OCCUR

SIMOULTANEOUSLY. SEE ALSO GENERAL INSTRUCTIONS DRAWINGS FOR FURTHER LOAD CONDITIONS.

LOADS

DEAD LOADS
SEE K07-01---0-H09-001 AND CORRESPONDING PLAN DRAWINGS.

LIVE	LOADS
ZONE	CAT

ZONE	CATEGORY	q _k kN/m²	Q _k kN	Ψ_0	Ψ_1	Ψ_2
100-01	E2	50	60	1,0	0,9	0,8
100-02	E2	50	60	1,0	0,9	0,8
100-03	E2	70	350	1,0	0,9	0,8
100-04	E2	150	300	1,0	0,9	0,8
100-05	E2	40	50	1,0	0,9	0,8
100-06	E2	5	5	1,0	0,9	0,8
100-07	В	2,5	3	0,7	0,5	0,3
110-01	E2	15	50	1,0	0,9	0,8
110-02	E2	15	50	1,0	0,9	0,8
110-03	E2	10	80	1,0	0,9	0,8
110 – 11	Н	0,4	1	0	0	0
115-01	E2	5	5	1,0	0,9	0,8
120-11	Н	0,4	1	0	0	0

SNOW LOAD

SEE K07-01---0-H09-001.

SNOW LOAD OCCUR ON ZONES 110-11 AND 120-11. FOR ZONE 110-11, SNOW CONCENTRATIONS SHALL BE CONSIDERED.

ATB 12K:

LOADS FROM ATB 12K OCCUR IN ZONE 100-02. LOADS FROM ATB 12K SHALL BE COMBINATED WITH LIVE LOADS AND LOADS FROM FORKLIFT.

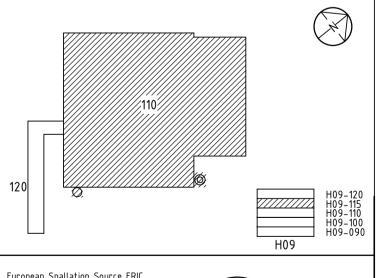
LOADS FROM FORKLIFT CAN OCCUR IN ANY POINT IN ZONE 100-01, 100-02 AND 100-04. LOADS FROM FORKLIFT SHALL BE COMBINATED WITH LIVE LOADS AND LOADS FROM ATB 12K.

EXCEPTIONAL INTERNAL WATER PRESSURE
WATER PRESSURE ON WALLS IN CASE OF INTERNAL FLOODING SHALL BEPPRISITER FOR TOND 1983. 01.09
ASSUMED MAXIMUM WATER LEVEL: 1500mm ABOVE FFL.

REV REVISION TYPE DATE SIGN

TECHNICAL	BASELINE

ESS CONVENTIONAL FACILITIES



European Spallation Source ERIC ESS, Tunavägen 24 P.O. Box 176 SE-221 00 Lund SWEDEN



DESIGN COORDINATOR	BUILDING OWNER
FRANK KEZERLE	BORIS KILDETOFT
ONSULTANT	PROJECT NUMBER
LAZ TVDÉNC AD	2176622

CONSULTANT		PROJECT NUMBER
K07 TYRÉNS A	AB	24766930
DRAWN BY	MANAGED BY	CHECKED BY
JMÅ	AAH	HNO
DATE	APPROVED BY	
	A.ABRAHAMSS	ON

H09 WASTE BUILDING

LOAD PLAN, LEVEL H09-115, ZONE 110

SCALE A1 1:100 A3 1:200

GENERAL INSTRUCTIONS ACC.TO K07-01---0-H09---001--003 BUILDING ELEMENTS ACC. TO K07-01---0-H09---011-012

EXPLANATIONS

ALL LOADS ARE GIVEN AS CHARACTERISTIC VALUES IF NOT STATED OTHERWISE.

LIVE LOADS TABLE BELOW SHOWS MAXIMUM ALLOWED UNIFORMLY DISTRIBUTED <u>OR</u> MAXIMUM ALLOWED

CONCENTRATED LOAD. LOAD REGULATION IS THAT THE UNIFORMLY DISTRIBUTED LOAD AND THE CONCENTRATED LOAD MUST NOT OCCUR

SIMOULTANEOUSLY. SEE ALSO GENERAL INSTRUCTIONS DRAWINGS FOR FURTHER LOAD CONDITIONS.

LOADS

DEAD LOADS

SEE K07-01---0-H09-001 AND CORRESPONDING PLAN DRAWINGS.

LIVE LOA	<u>ADS</u>
ZONE	CATEG

ZONE	CATEGORY	q _k kN/m²	${f Q}_{\bf k}$ kN	Ψ_0	Ψ ₁	Ψ_2
100-01	E2	50	60	1,0	0,9	0,8
100-02	E2	50	60	1,0	0,9	0,8
100-03	E2	70	350	1,0	0,9	0,8
100-04	E2	150	300	1,0	0,9	0,8
100-05	E2	40	50	1,0	0,9	0,8
100-06	E2	5	5	1,0	0,9	0,8
100-07	В	2,5	3	0,7	0,5	0,3
110-01	E2	15	50	1,0	0,9	0,8
110-02	E2	15	50	1,0	0,9	0,8
110-03	E2	10	80	1,0	0,9	0,8
110 – 11	Н	0,4	1	0	0	0
115-01	E2	5	5	1,0	0,9	0,8
120-11	Н	0,4	1	0	0	0

SNOW LOAD

SEE K07-01---0-H09-001.

SNOW LOAD OCCUR ON ZONES 110-11 AND 120-11. FOR ZONE 110-11, SNOW CONCENTRATIONS SHALL BE CONSIDERED.

ATB 12K:

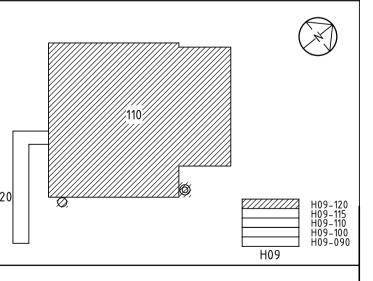
LOADS FROM ATB 12K OCCUR IN ZONE 100-02. LOADS FROM ATB 12K SHALL BE COMBINATED WITH LIVE LOADS AND LOADS FROM FORKLIFT.

LOADS FROM FORKLIFT CAN OCCUR IN ANY POINT IN ZONE 100-01, 100-02 AND 100-04. LOADS FROM FORKLIFT SHALL BE COMBINATED WITH LIVE LOADS AND LOADS FROM ATB 12K.

EXCEPTIONAL INTERNAL WATER PRESSURE
WATER PRESSURE ON WALLS IN CASE OF INTERNAL FLOODING SHALL BEPTONSITE FOR TON 0483.01.09
ASSUMED MAXIMUM WATER LEVEL: 1500mm ABOVE FFL

REV REVISION TYPE DATE SIGN TECHNICAL BASELINE

ESS CONVENTIONAL FACILITIES



European Spallation Source ERIC ESS, Tunavägen 24 P.O. Box 176 SE-221 00 Lund SWEDEN

EUROPEAN SPALLATION SOURCE

DESIGN COORDINATOR
FRANK KEZERLE
CONSULTANT
K07 TYRÉNS AB

BUILDING OWNER

BORIS KILDETOFT

PROJECT NUMBER

24766930

CHECKED BY

HNO MANAGED BY

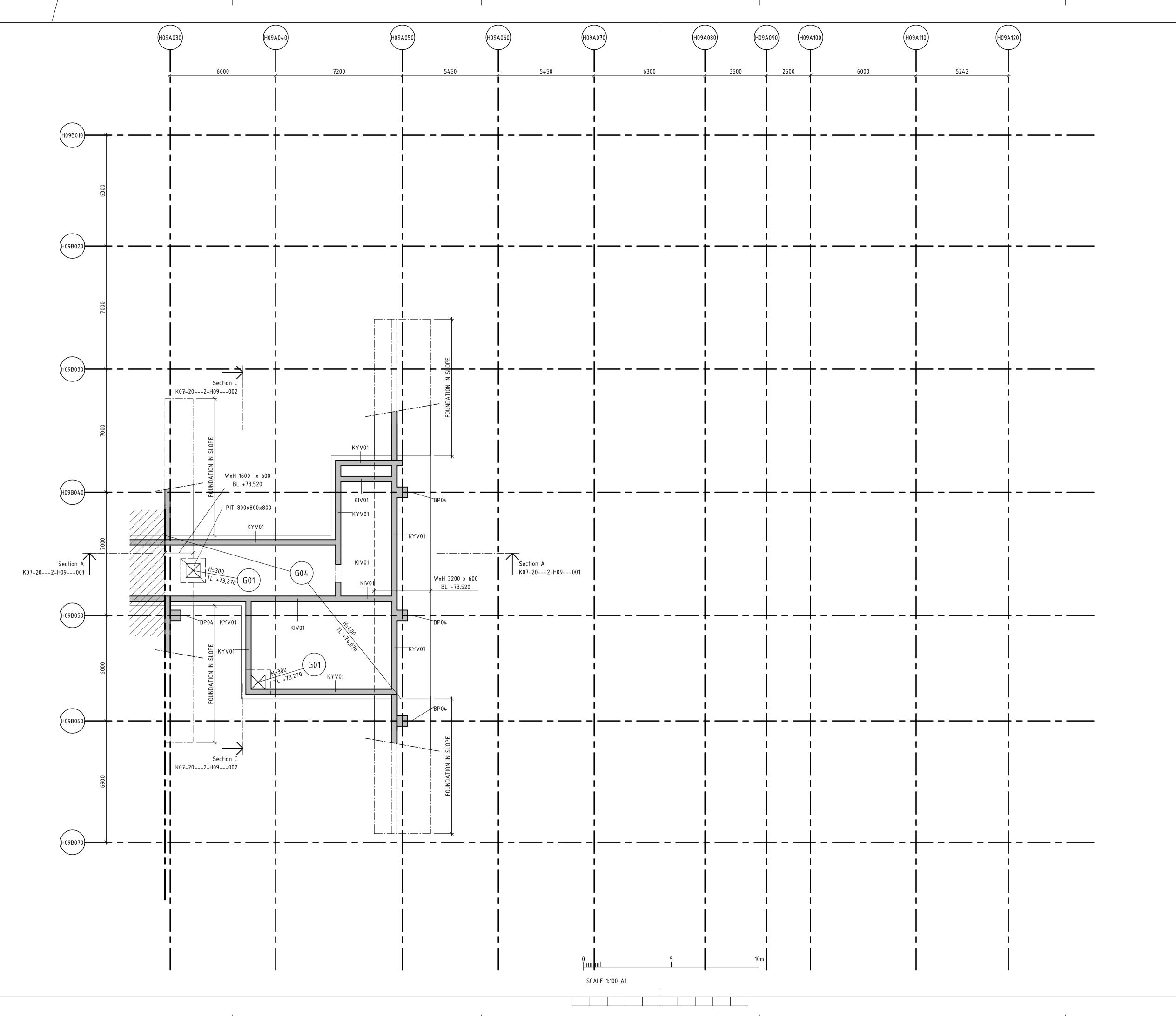
AAH APPROVED BY

A.ABRAHAMSSON

H09 WASTE BUILDING

LOAD PLAN, LEVEL H09-120, ZONE 110

SCALE A1 1:100 A3 1:200



GENERAL INSTRUCTIONS ACCORDING TO K07-01---0-H09---001--003

BUILDING ELEMENTS ACCORDING TO K07-01---0-H09---011-012

EXPLANATIONS

TL TOP LEVEL
BL BOTTOM LEVEL
WxH WIDTH x HEIGTH
WxD WIDTH x DEPTH
FC FINISHED CONCRETE

PD DRAFT 2018-01-09

	IL VISIO	N TYPE		DATE SIGN
	TECH	NICAL	BAS	ELINE
ESS		ENTIO ARY BUIL		FACILITIES NORTH
		110		
120			Ø	H09-120 H09-115 H09-110 H09-090 H09
European S ESS, Tunav P.O. Box 17 SE-221 00 SWEDEN www.esss.s	6 Lund	ERIC		EUROPEAN SPALLATION SOURCE
DESIGN COOL FRANK	KEZERI	_E	BUILDING OWI	NER KILDETOFT PROJECT NUMBER
	YRÉNS A			24766930
		MANAGED BY		CHECKED BY
DRAWN BY		AAH		l HNO

FOUNDATION PLAN, LEVEL 090, ZONE 110

SCALE
A1 1:100
A3 1:200

DRAWING NUMBER

K07-15---1-H09090110

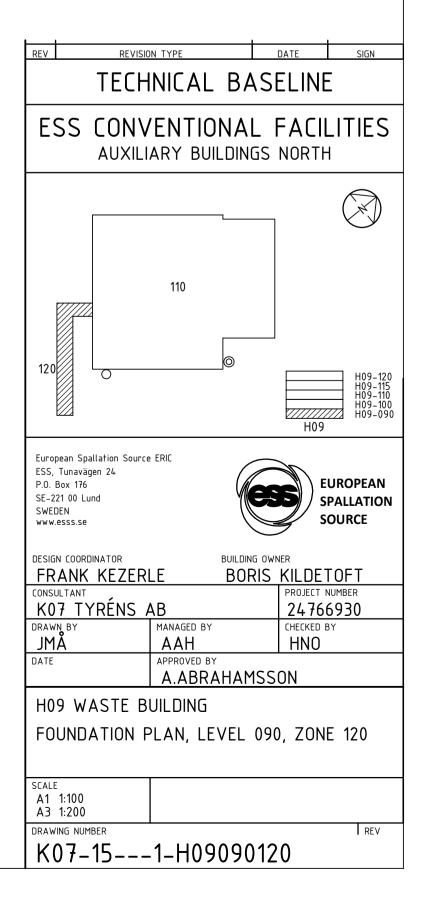
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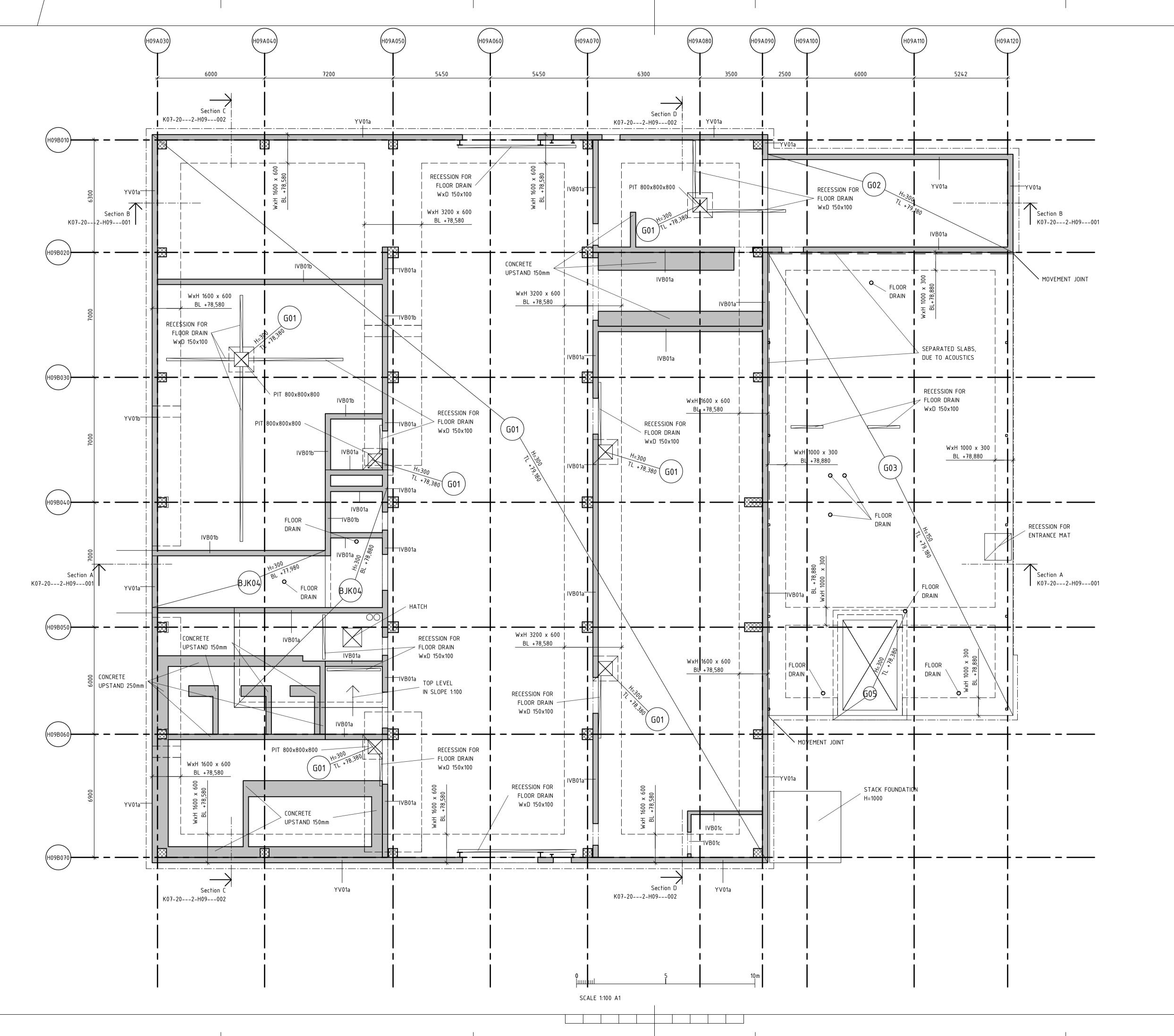
GENERAL INSTRUCTIONS ACCORDING
TO K07-01---0-H09---001--003

BUILDING ELEMENTS ACCORDING
TO K07-01---0-H09---011-012

EXPLANATIONS

TL TOP LEVEL
BL BOTTOM LEVEL
WxH WIDTH x HEIGTH
WxD WIDTH x DEPTH
FC FINISHED CONCRETE



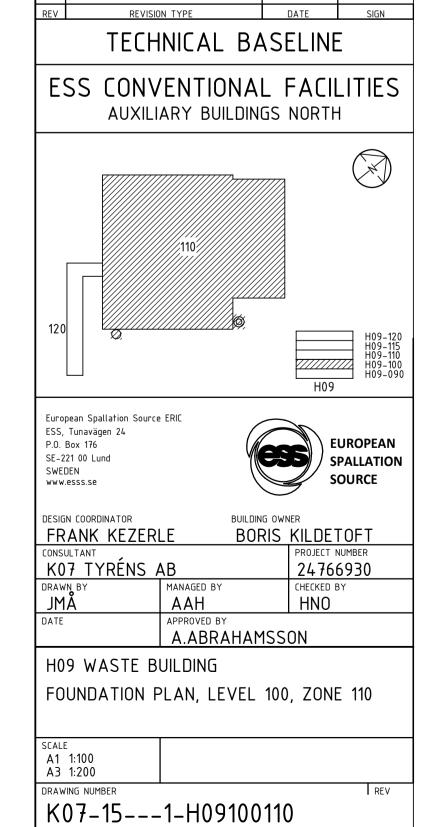


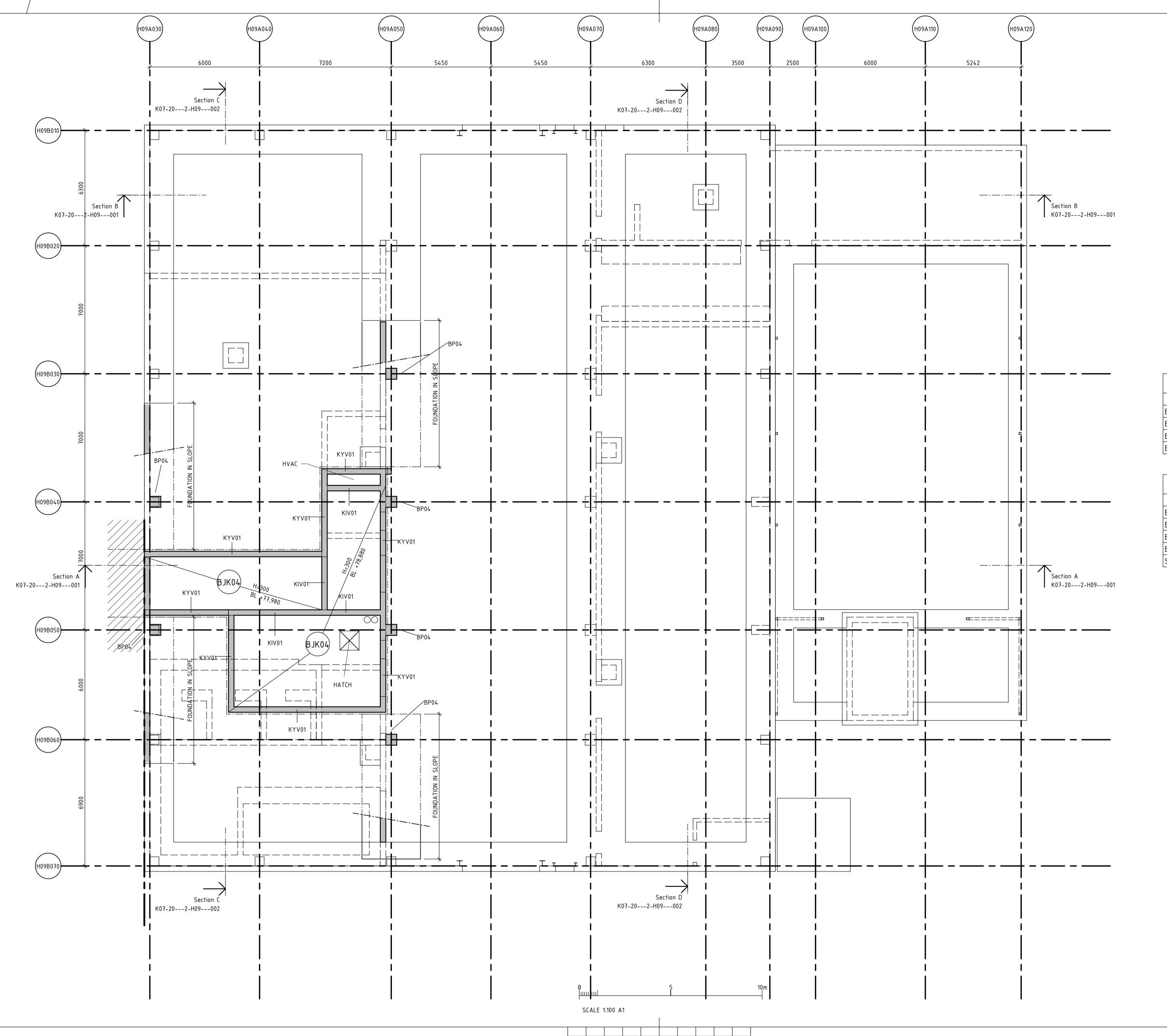
GENERAL INSTRUCTIONS ACCORDING TO K07-01---0-H09---001--003

BUILDING ELEMENTS ACCORDING TO K07-01---0-H09---011-012

EXPLANATIONS

TL TOP LEVEL
BL BOTTOM LEVEL
WxH WIDTH x HEIGTH
WxD WIDTH x DEPTH
FC FINISHED CONCRETE





GENERAL INSTRUCTIONS ACCORDING TO K07-01---0-H09---001--003

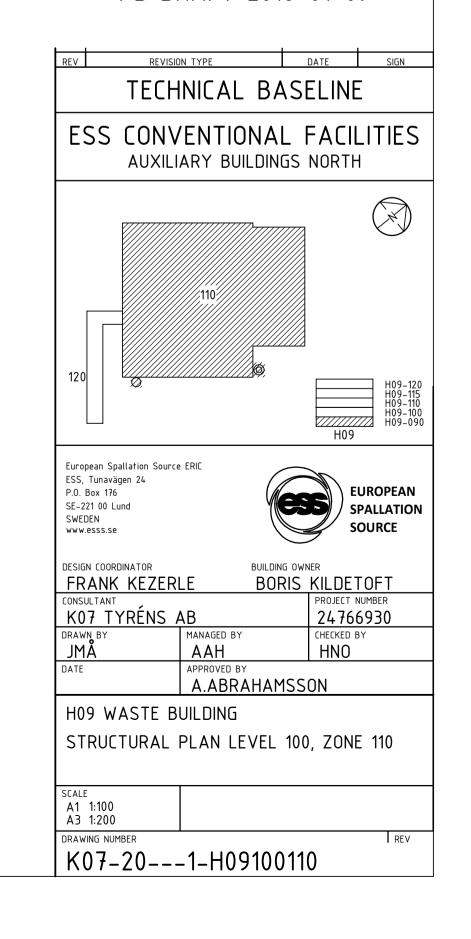
BUILDING ELEMENTS ACCORDING TO K07-01---0-H09---011-012

EXPLANATIONS

TL TOP LEVEL
BL BOTTOM LEVEL
WxH WIDTH x HEIGTH
WxD WIDTH x DEPTH
FC FINISHED CONCRETE

BEAMS						
MARKING	MATERIAL	TYPE	COUNT			
BB01	CONCRETE PRECAST	FB/FH 55/70	10			
BB02	CONCRETE PRECAST	FB/FH 55/50	12			
BB03	CONCRETE PRECAST	FB/FH 45/60	12			
BB04	CONCRETE PRECAST	FB/F 70/70	12			

COLUMNS			
MARKING	MATERIAL	TYPE	COUNT
BP01	CONCRETE PRECAST	600x600	20
BP02	CONCRETE PRECAST	500x500	38
BP03	CONCRETE PRECAST	1000×500	4
BP04	CONCRETE	600x600	7
SP01	STEEL	VKR120x120x8	10



SCALE 1:100 A1

REFERENCES

GENERAL INSTRUCTIONS ACCORDING TO K07-01---0-H09---001--003

BUILDING ELEMENTS ACCORDING

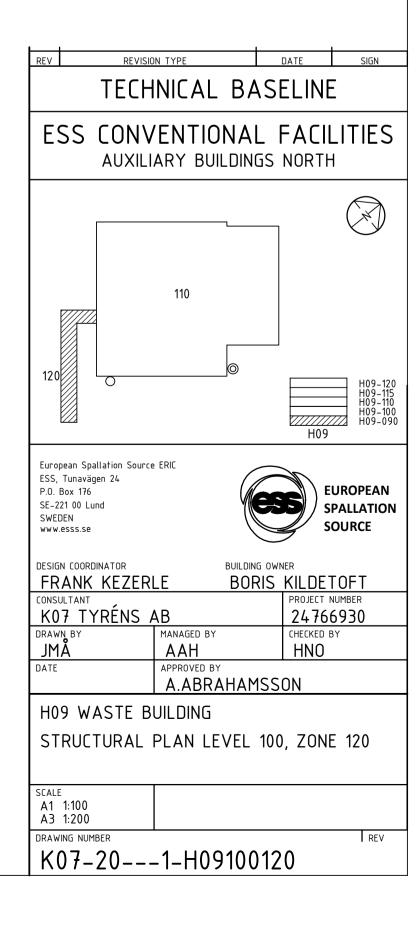
TO K07-01---0-H09---011-012

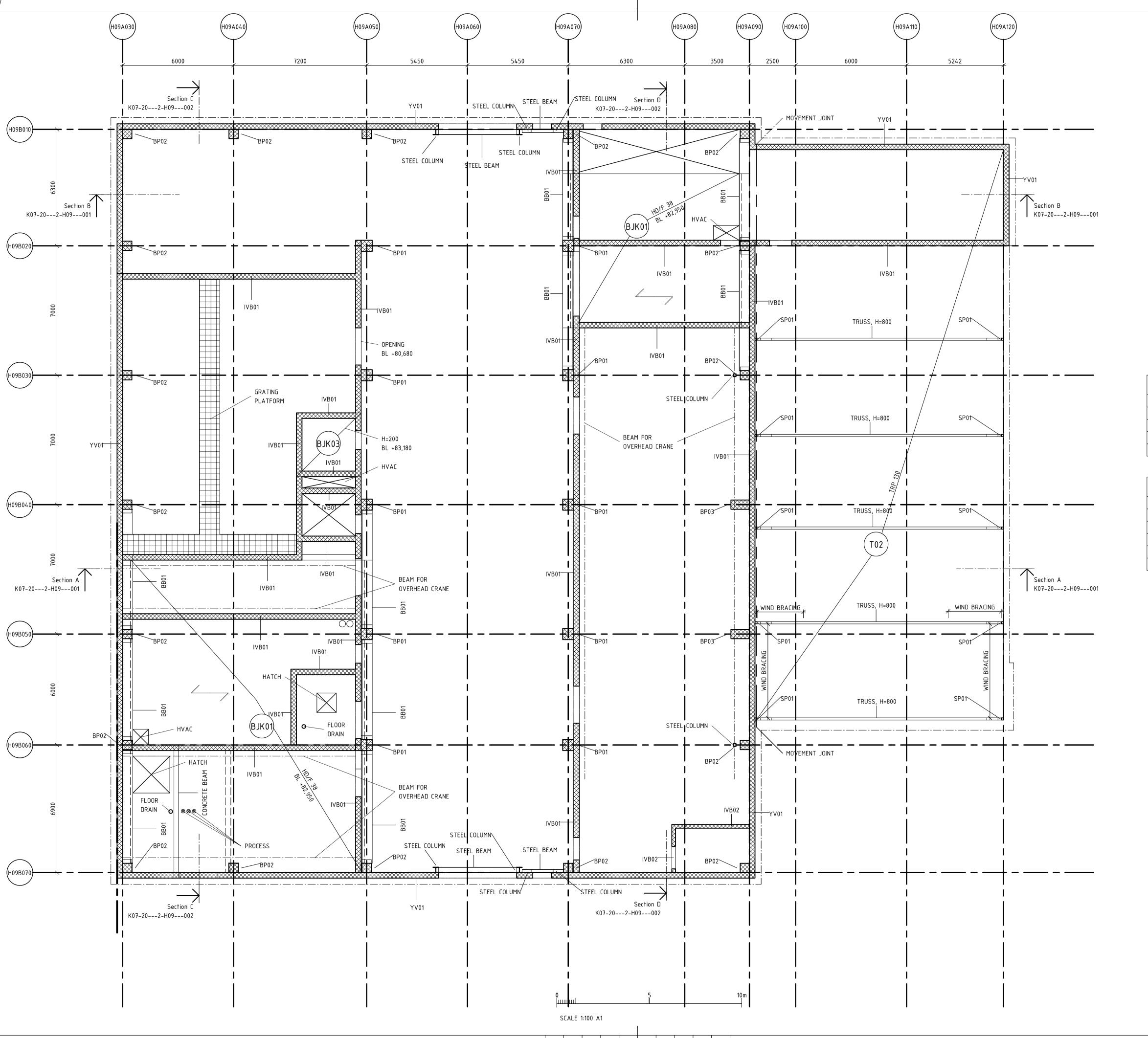
EXPLANATIONS

TL TOP LEVEL
BL BOTTOM LEVEL
WxH WIDTH x HEIGTH
WxD WIDTH x DEPTH
FC FINISHED CONCRETE

BEAMS			
MARKING	MATERIAL	TYPE	COUNT
BB01	CONCRETE PRECAST	FB/FH 55/70	10
BB02	CONCRETE PRECAST	FB/FH 55/50	12
BB03	CONCRETE PRECAST	FB/FH 45/60	12
BB04	CONCRETE PRECAST	FB/F 70/70	12

COLUMNS			
MARKING	MATERIAL	TYPE	COUNT
BP01	CONCRETE PRECAST	600x600	20
BP02	CONCRETE PRECAST	500x500	38
BP03	CONCRETE PRECAST	1000×500	4
BP04	CONCRETE	600x600	7
SP01	STEEL	VKR120x120x8	10





GENERAL INSTRUCTIONS ACCORDING TO K07-01---0-H09---001--003

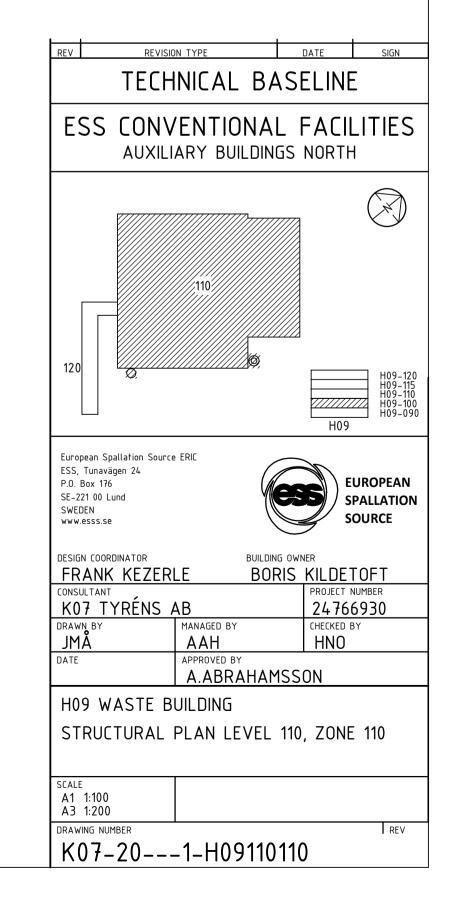
BUILDING ELEMENTS ACCORDING TO K07-01---0-H09---011-012

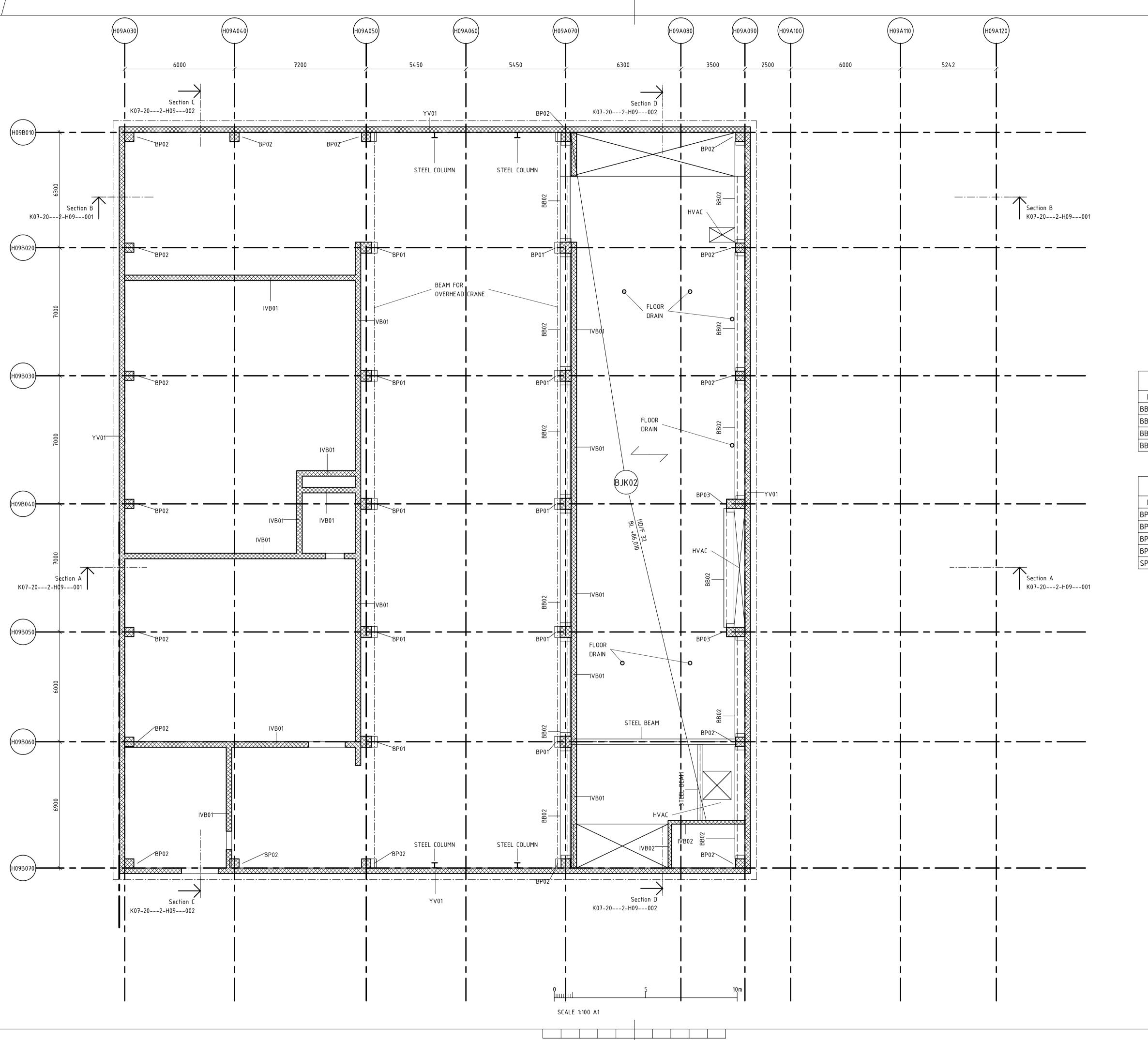
EXPLANATIONS

TL TOP LEVEL
BL BOTTOM LEVEL
WxH WIDTH x HEIGTH
WxD WIDTH x DEPTH
FC FINISHED CONCRETE

BEAMS			
MARKING	MATERIAL	TYPE	COUNT
BB01	CONCRETE PRECAST	FB/FH 55/70	10
BB02	CONCRETE PRECAST	FB/FH 55/50	12
BB03	CONCRETE PRECAST	FB/FH 45/60	12
BB04	CONCRETE PRECAST	FB/F 70/70	12

COLUMNS			
MARKING	MATERIAL	TYPE	COUNT
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BP02	CONCRETE PRECAST	500x500	38
BP03	CONCRETE PRECAST	1000×500	4
BP04	CONCRETE	600x600	7
SP01	STEEL	VKR120x120x8	10





GENERAL INSTRUCTIONS ACCORDING TO K07-01---0-H09---001--003

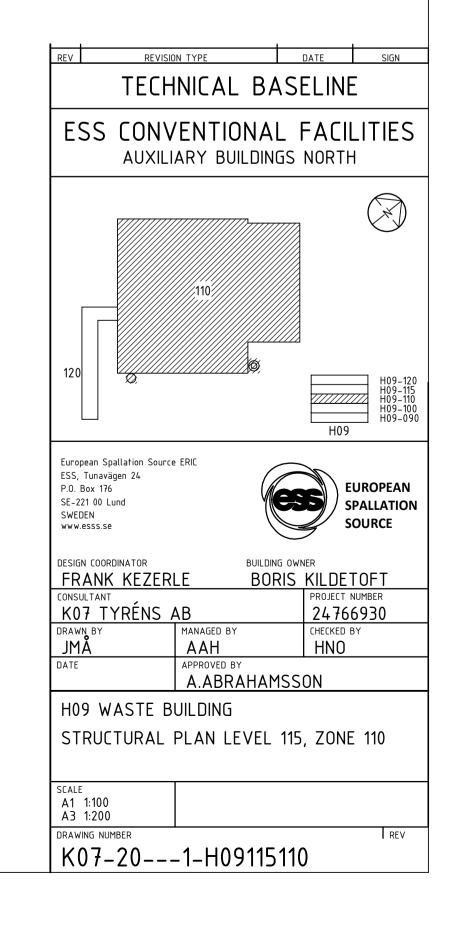
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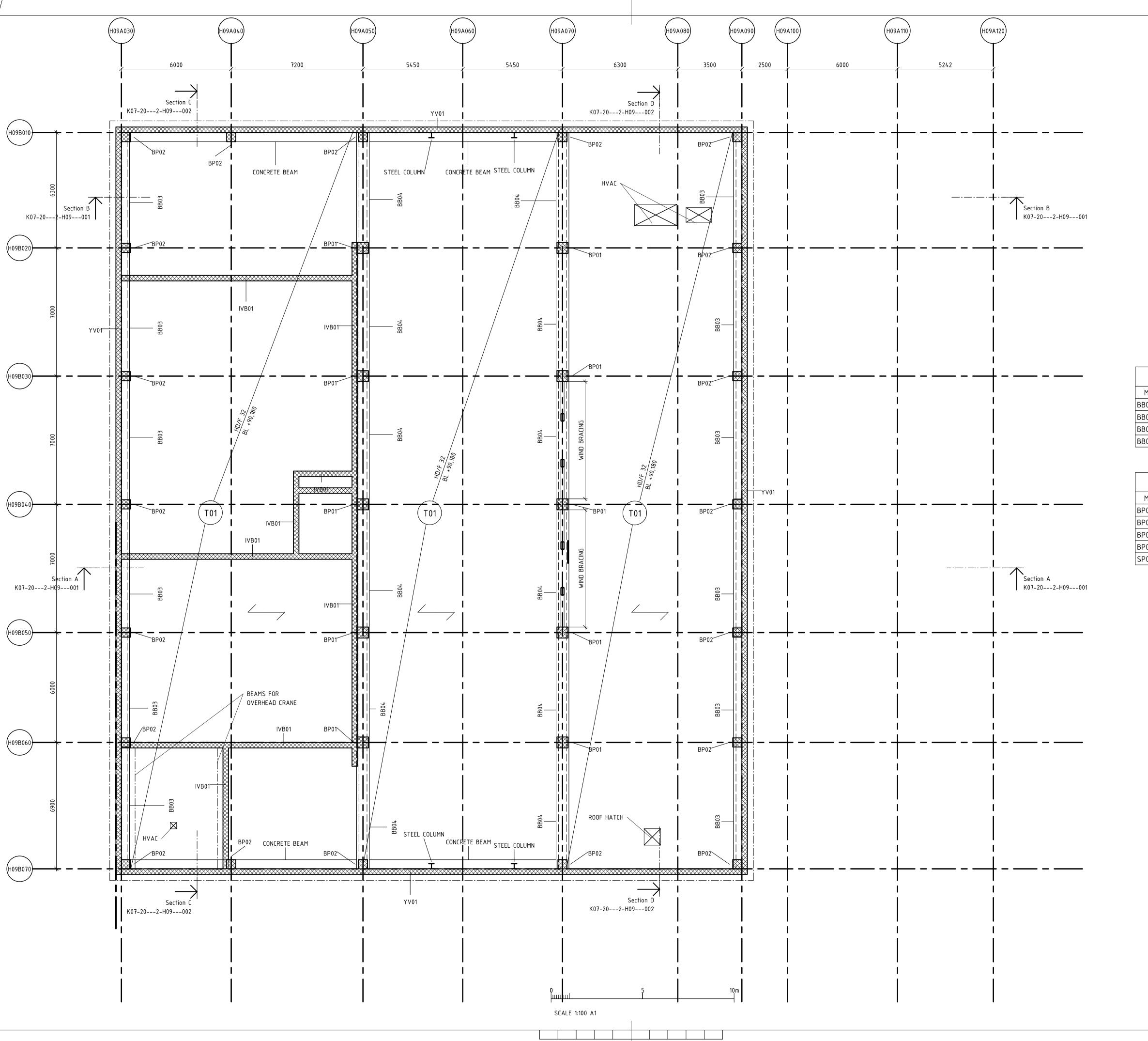
EXPLANATIONS

TL TOP LEVEL
BL BOTTOM LEVEL
WxH WIDTH x HEIGTH
WxD WIDTH x DEPTH
FC FINISHED CONCRETE

BEAMS			
MARKING	MATERIAL	TYPE	COUNT
BB01	CONCRETE PRECAST	FB/FH 55/70	10
BB02	CONCRETE PRECAST	FB/FH 55/50	12
BB03	CONCRETE PRECAST	FB/FH 45/60	12
BB04	CONCRETE PRECAST	FB/F 70/70	12

COLUMNS			
MARKING	MATERIAL	TYPE	COUNT
BP01	CONCRETE PRECAST	600x600	20
BP02	CONCRETE PRECAST	500x500	38
BP03	CONCRETE PRECAST	1000×500	4
BP04	CONCRETE	600x600	7
SP01	STEEL	VKR120x120x8	10





GENERAL INSTRUCTIONS ACCORDING TO K07-01---0-H09---001--003

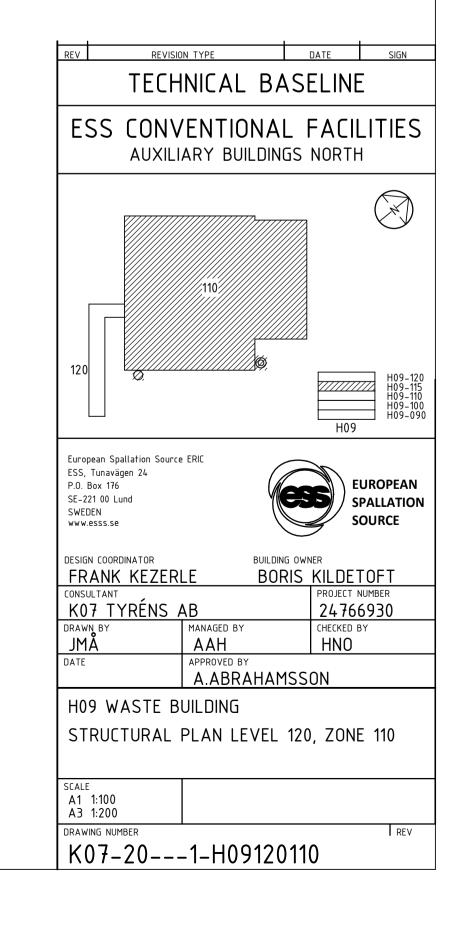
BUILDING ELEMENTS ACCORDING
TO K07-01---0-H09---011-012

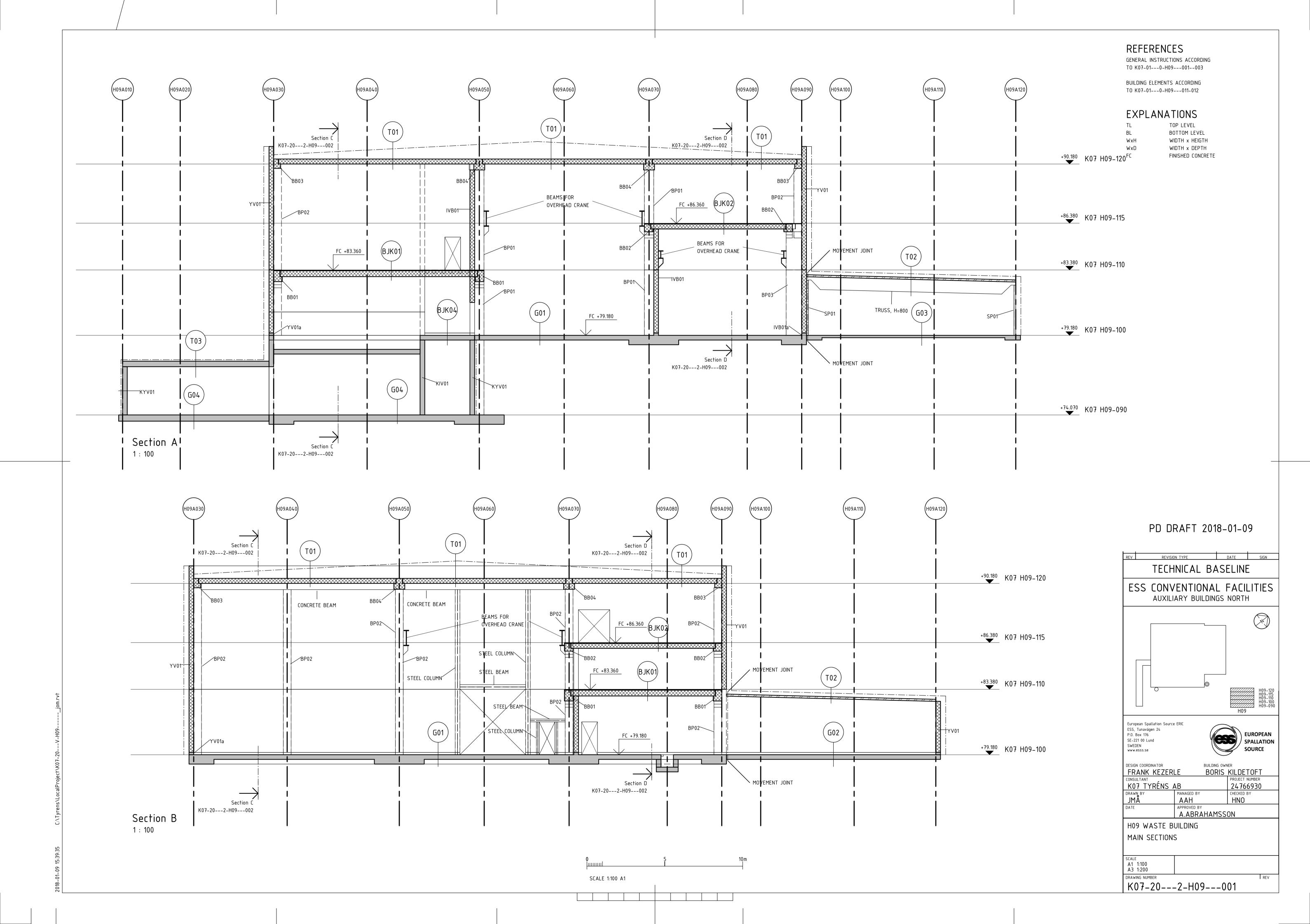
EXPLANATIONS

TL TOP LEVEL
BL BOTTOM LEVEL
WxH WIDTH x HEIGTH
WxD WIDTH x DEPTH
FC FINISHED CONCRETE

BEAMS			
MARKING	MATERIAL	TYPE	COUNT
BB01	CONCRETE PRECAST	FB/FH 55/70	10
BB02	CONCRETE PRECAST	FB/FH 55/50	12
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COLUMNS			
MARKING	MATERIAL	TYPE	COUNT
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BP04	CONCRETE	600x600	7
SP01	STEEL	VKR120x120x8	10





SCALE 1:100 A1

REFERENCES

GENERAL INSTRUCTIONS ACCORDING TO K07-01---0-H09---001--003

BUILDING ELEMENTS ACCORDING

EXPLANATIONS

TO K07-01---0-H09---011-012

TL TOP LEVEL
BL BOTTOM LEVEL
WxH WIDTH x HEIGTH
WxD WIDTH x DEPTH
FC FINISHED CONCRETE

